

Legend

- Site Boundary
- Residential - Lower Density
- Residential - Medium Density
- Mixed Use - Neighbourhood Centre
- Primary School
- Potential Allocation of Willabrant
- Proposed Key Frontage
- Potential Landmark Building
- Children's Play Area & Walking Zone - 100m
- Proposed Public Open Space
- Existing Trees/Planting
- Proposed Tree
- Existing Water Feature
- Road Plan
- Potential Vehicle Access Point
- Potential Pedestrian/Cycle Access Point
- Principal Access Route
- Minor Access Route

date

Padbrook Park Golf Course

Amendment
Concept Masterplan

Job Title

Job No/Drawing No

Scale

Date

Drawn by

CP

Masterplan 2

Do not scale

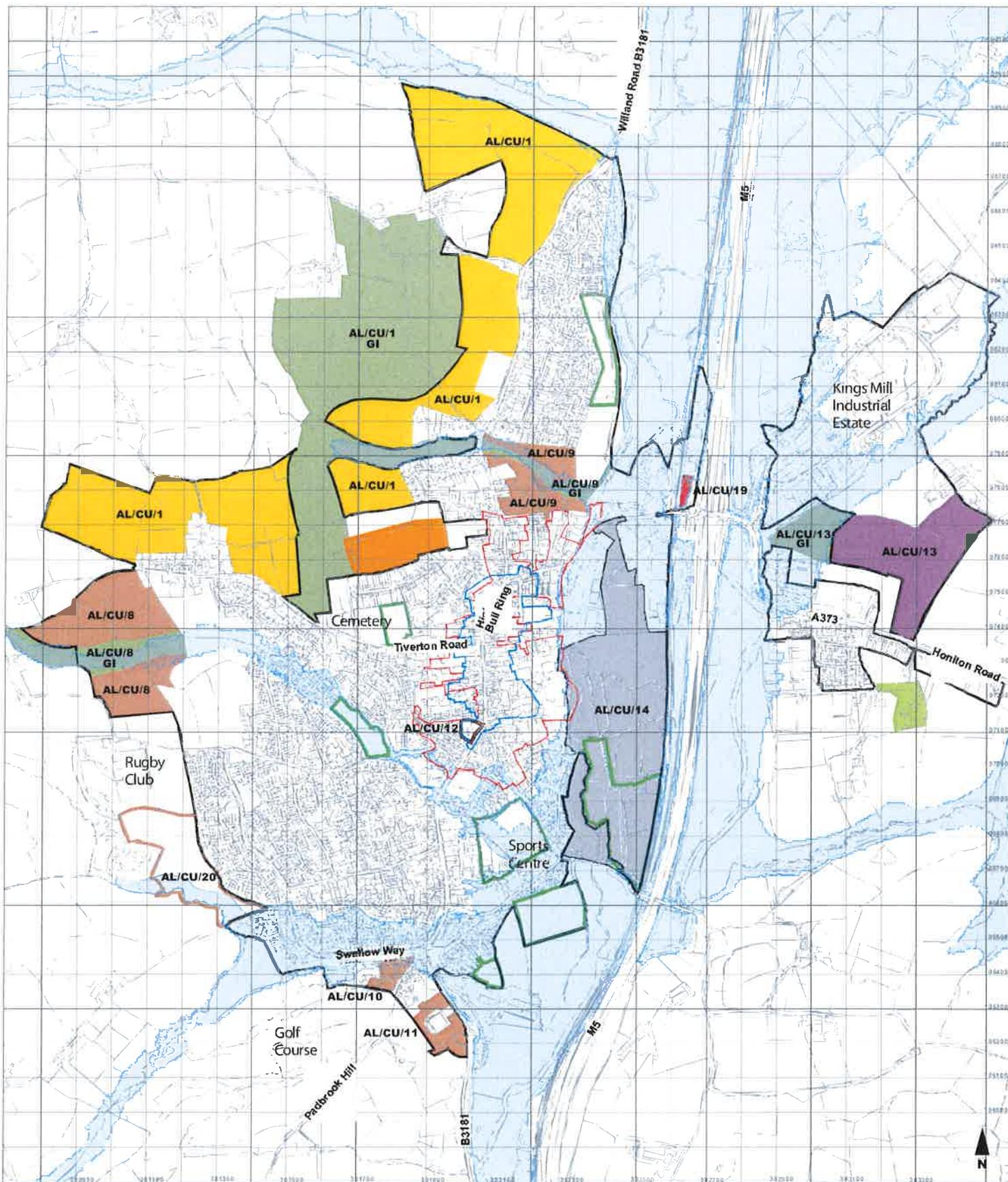
All dimensions to be checked on site

pad Design Ltd - 10 Osborne Road - Southville - 61051 B31 TRK - 0117 9310097

pad

APPENDIX B

LDP ALLOWCATION PLAN



- Settlement Limit
- Residential
- Contingency Housing Site
- Mixed Use
- Green Infrastructure
- Employment
- Town Centre Boundary

-
-
-
-
-
-
-

- Community Use
- Railway Station Car Park
- Area for Eastern Relief Road
- Conservation Area
- Important Land for Sport and Recreation
- Ancient Monument
- County Wildlife Site
- Floodplain

-
-
-
-
-
-
-

Mid Devon Local Development Framework

Proposals Map
Cullompton

Scale
1:10,000

Note: The Devon County Council has updated the base to show recent changes. Consequently, not all the elements may be as shown. The map is based on Ordnance Survey data and is not a guarantee of accuracy. The map is for information only and should not be used for any other purpose. © 2011 Devon County Council. All rights reserved.

January
2011

APPENDIX C

PUBLIC TRANSPORT INFORMATION

CULLOMPTON TOWN CIRCULAR

CULLOMPTON - KENTISBEARE

DARTLINE 350
DARTLINE 350

ROUTE: CROW BRIDGE, Shortlands Road, Tiverton Road, Langlands Road, Maintoba Gardens, Orchard Way, Colebrooke Lane, Swallow Way, Fulford Drive, Exeter Road, Meadow Lane, Sports Centre, Duke Street, Chestnut Avenue, Rivermead, Duke Street, Meadow Lane, Exeter Road, Exeter Hill, Fore Street, High Street, Higher Street, Willand Road, Saxon Way, Norman Drive, Millenium Way, Tesco, Station Road, A373, Post Cross, Kentisbeare.
Returns via Parsons Close

Monday - Friday (Except Public Holidays)

CROW BRIDGE	0930	1030	1131
LANGLANDS ROAD	0933	1033	1134
CULM VALLEY SPORTS CENTRE	0937	1037	1138
CULLOMPTON, Memorial	0944	1044	1145
HEALTH CENTRE	0946	1046	1147
TESCO	0951	1051	1152
KENTISBEARE, Post Office	0959	-	1200
KENTISBEARE, Parsons Close	1000	-	1201

KENTISBEARE, Post Office	0959	-	1200
KENTISBEARE, Parsons Close	1000	-	1201
TESCO	1008	1109	1209
HEALTH CENTRE	1013	1114	1214
CULLOMPTON, Market House	1016	1117	1217
CULM VALLEY SPORTS CENTRE	R	R	R
LANGLANDS ROAD	1024	1125	1225
CROW BRIDGE	1029	1130	1230

CODE: R - CALLS TO SET DOWN ON REQUEST BY PASSENGERS ON THE BUS

NOTE:  - MOST JOURNEYS ARE NORMALLY OPERATED BY LOW FLOOR FULLY ACCESSIBLE VEHICLES

Thursday only (Except Public Holidays)

HOLCOMBE ROGUS, Whitebrook Terrace	0905	EXETER, Bus Station Stand 1	1330
HOLCOMBE ROGUS, Prince Of Wales	0907	PINHOE, Lloyds Bank	1341
WESTLEIGH, Shelter	0915	POLTIMORE, Post Box	1346
BURLESCOMBE, Ayshford Arms	0919	ELLERHAYES	1357
CULMSTOCK, Millmoor	0928	BRADNINCH, Guildhall	1405
CRADDOCK, Phone Box	0933	CULLOMPTON, War Memorial	1412
ASHILL, Phone Box	0936	KENTISBEARE, Parsons Close	1421
KENTISBEARE, Post Office	0946	KENTISBEARE, Post Office	1422
KENTISBEARE, Parsons Close	0947	ASHILL, Phone Box	1432
CULLOMPTON, Market House	0957	CRADDOCK, Phone Box	1435
BRADNINCH, Guildhall	1004	CULMSTOCK, Millmoor	1440
ELLERHAYES	1012	BURLESCOMBE, Ayshford Arms	1449
POLTIMORE, Post Box	1023	WESTLEIGH, Shelter	1453
PINHOE, Heart Of Oak	1028	HOLCOMBE ROGUS, Prince Of Wales	1501
EXETER, Bus Station Stand 1	1039	HOLCOMBE ROGUS, Whitebrook Terrace	1503

This service is financially supported by Devon County Council

HONITON - CULLOMPTON Via Kentisbeare
CULLOMPTON - HONITON Via Plymtree, Feniton

STAGECOACH 694
STAGECOACH 694

Monday - Saturday (Except Public Holidays)

	NF	F		NF	F
HONITON, Dowell Street Car Park	0845	0845	HONITON, Congregational Church	1215	1215
AWLISCOMBE, Nap View	0850	0850	HONITON, Job Centre	1217	1217
DULFORD, Phone Box	0900	0900	WESTON, Otter Inn	-	1225
KENTISBEARE, Post Office	0905	0905	BUCKERELL, Church	-	1229
KENTISBEARE, Parsons Close	0906	0906	FENITON, Village Green	1229	1234
CULLOMPTON, Market House	0915	0915	FENITON, Wells Avenue	1232	1237
WESTCOTT, Merry Harriers	0922	0922	COLESTOCKS, Moor View	1237	1242
LANGFORD GREEN	0925	0925	PAYHEMBURY, Stores	1239	1244
PLYMTREE, Church	0930	0930	LUTON, Post Box	1242	1247
NORMANS GREEN, Shelter	0932	0932	NORMANS GREEN, Shelter	1250	1255
LUTON, Post Box	0940	0940	PLYMTREE, Church	1252	1257
PAYHEMBURY, Stores	0943	0943	LANGFORD GREEN	1257	1302
COLESTOCKS, Moor View	0945	0945	WESTCOTT, Merry Harriers	1300	1305
FENITON, Wells Avenue	0950	0950	CULLOMPTON, War Memorial	1310	1315
FENITON, Village Green	0954	0954	KENTISBEARE, Parsons Close	1319	1324
BUCKERELL, Church	-	1001	KENTISBEARE, Post Office	1320	1325
WESTON, Otter Inn	-	1006	DULFORD, Phone Box	1325	1330
HONITON, Lace Walk	1008	1018	AWLISCOMBE, Nap View	1335	1340
HONITON, Red Cow	1010	1020	HONITON, Lace Walk	1340	1345

CODE: F - FRIDAY ONLY NF - NOT FRIDAY

NOTE:  - MOST JOURNEYS ARE NORMALLY OPERATED BY LOW FLOOR FULLY ACCESSIBLE VEHICLES

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APPENDIX D

ARCADY RESULTS FOR SWALLOW WAY/EXETER ROAD ROUNDABOUT

Junctions 8
ARCADY 8 - Roundabout Module
Version: 8.0.2.316 [14 Feb 2013] © Copyright TRL Limited, 2013
For sales and distribution information, program advice and maintenance, contact TRL: Tel: +44 (0)1344 770758 E-mail: software@trl.co.uk Web: http://www.trlsoftware.co.uk
The users of this computer program for the solution of an engineering problem are in no way relieved of their responsibility for the correctness of the solution

Filename: Exeter Road-Swallow Way.arc8
Path: S:\C9841 - Colebrooke Lane, Cullompton\Calculations\Traffic\Arcady
Report generation date: 25/06/2013 15:07:59

- » Existing Layout - Base Year 2013, AM
- » Existing Layout - Base Year 2013, PM
- » Existing Layout - Testing Year 2023 + Dev, AM Peak
- » Existing Layout - Testing Year 2023 + Dev, PM Peak

File summary

File Description

Title	Exeter Road/Swallow Way Roundabout
Location	Cullompton
Site Number	C9841 - Colebrooke Lane, Cullompton
Date	18/06/2012
Version	Preliminary
Status	Existing Layout
Identifier	SR
Client	Genesis Town Planning
Jobnumber	C9841
Enumerator	CARDIFF\sradford
Description	

Analysis Options

Vehicle Length (m)	Do Queue Variations	Calculate Residual Capacity	Residual Capacity Criteria Type	RFC Threshold	Average Delay Threshold (s)	Queue Threshold (PCU)
5.75	✓		N/A	0.85	36.00	20.00

Units

Distance Units	Speed Units	Traffic Units Input	Traffic Units Results	Flow Units	Average Delay Units	Total Delay Units	Rate Of Delay Units
m	kph	Veh	Veh	perHour	s	-Min	perMin

Existing Layout - Base Year 2013, AM

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Geometry	Arm 4 - Roundabout Geometry	Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution.

Analysis Set Details

Name	Roundabout Capacity Model	Description	Include In Report	Use Specific Demand Set(s)	Specific Demand Set(s)	Locked	Network Flow Scaling Factor (%)	Network Capacity Scaling Factor (%)	Reason For Scaling Factors
Existing Layout	ARCADY		✓				100.000	100.000	

Demand Set Details

Name	Scenario Name	Time Period Name	Description	Traffic Profile Type	Model Start Time (HH:mm)	Model Finish Time (HH:mm)	Model Time Period Length (min)	Time Segment Length (min)	Results For Central Hour Only	Single Time Segment Only	Locked	Run Automatically	Use Relationship	Relationship
Base Year 2013, AM	Base Year 2013	AM		ONE HOUR	07:45	09:15	90	15				✓		

Junction Network

Junctions

Name	Junction Type	Arm Order	Grade Separated	Large Roundabout	Do Geometric Delay	Junction Delay (s)	Junction LOS
(untitled)	Roundabout	1,2,3,4				2.42	A

Junction Network Options

Driving Side	Lighting
Left	Normal/unknown

Arms

Arms

Arm	Name	Description
1	Exeter Road (North)	
2	Exeter Road (South)	
3	Bowhill	
4	Swallow Way	

Capacity Options

Arm	Minimum Capacity (PCU/hr)	Maximum Capacity (PCU/hr)	Assume Flat Start Profile	Initial Queue (PCU)
1	0.00	99999.00		0.00
2	0.00	99999.00		0.00
3	0.00	99999.00		0.00
4	0.00	99999.00		0.00

Roundabout Geometry

Arm	V - Approach road half-width (m)	E - Entry width (m)	Γ - Effective flare length (m)	R - Entry radius (m)	D - inscribed circle diameter (m)	PHI - Conflict (entry) angle (deg)	Exit Only
1	7.00	8.00	30.00	37.50	30.00	60.00	
2	4.50	8.00	20.00	12.50	30.00	50.00	
3	3.00	8.00	25.00	10.00	30.00	60.00	
4	3.00	7.00	35.00	62.50	30.00	60.00	

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Pedestrian Crossings

Arm	Crossing Type
1	None
2	None
3	None
4	None

Slope / Intercept / Capacity

Roundabout Slope and Intercept used in model

Arm	Enter slope and intercept directly	Entered slope	Entered Intercept (PCU/hr)	Final Slope	Final Intercept (PCU/hr)
1		(calculated)	(calculated)	0.735	2200.146
2		(calculated)	(calculated)	0.656	1841.551
3		(calculated)	(calculated)	0.580	1552.365
4		(calculated)	(calculated)	0.630	1669.173

The slope and intercept shown above include any corrections and adjustments

Traffic Flows

Demand Set Data Options

Default Vehicle Mix	Vehicle Mix Varies Over Time	Vehicle Mix Varies Over Turn	Vehicle Mix Varies Over Entry	Vehicle Mix Source	PCU Factor for a HV (PCU)	Default Turning Proportions	Estimate from entry/exit counts	Turning Proportions Vary Over Time	Turning Proportions Vary Over Turn	Turning Proportions Vary Over Entry
		✓	✓	HV Percentages	2.00				✓	✓

Entry Flows

General Flows Data

Arm	Profile Type	Use Turning Counts	Average Demand Flow (Veh/hr)	Flow Scaling Factor (%)
1	ONE HOUR	✓	276.00	100.000
2	ONE HOUR	✓	198.00	100.000
3	ONE HOUR	✓	120.00	100.000
4	ONE HOUR	✓	159.00	100.000

Turning Proportions

Turning Counts or Proportions (Veh/hr) - Junction 1 (for whole period)

		To			
		1	2	3	4
From	1	0.000	175.000	73.000	28.000
	2	175.000	0.000	8.000	15.000
	3	106.000	2.000	0.000	12.000
	4	63.000	70.000	26.000	0.000

Turning Proportions (Veh) - Junction 1 (for whole period)

		To			
		1	2	3	4
From	1	0.00	0.63	0.26	0.10
	2	0.88	0.00	0.04	0.08
	3	0.88	0.02	0.00	0.10
	4	0.40	0.44	0.16	0.00

Vehicle Mix

Average PCU Per Vehicle - Junction 1 (for whole period)

		To			
		1	2	3	4
From	1	1.000	1.015	1.000	1.031
	2	1.039	1.000	1.000	1.027
	3	1.010	1.000	1.000	1.000
	4	1.000	1.000	1.000	1.000

Heavy Vehicle Percentages - Junction 1 (for whole period)

		To			
		1	2	3	4
From	1	0.000	1.500	0.000	3.100
	2	3.900	0.000	0.000	2.700
	3	1.000	0.000	0.000	0.000
	4	0.000	0.000	0.000	0.000

Results

Results Summary for whole modelled period

Arm	Max RFC	Max Delay (s)	Max Queue (Veh)	Max LOS	Average Demand (Veh/hr)	Total Junction Arrivals (Veh)	Total Queuing Delay (Veh-min)	Average Queuing Delay (s)	Rate Of Queuing Delay (Veh-min/min)	Inclusive Total Queuing Delay (Veh-min)	Inclusive Average Queuing Delay (s)
1	0.15	2.01	0.17	A	253.26	379.69	12.32	1.95	0.14	12.32	1.95
2	0.13	2.45	0.15	A	181.69	272.53	10.77	2.37	0.12	10.77	2.37
3	0.09	2.85	0.10	A	110.11	165.17	7.58	2.75	0.08	7.58	2.75
4	0.12	2.78	0.14	A	145.90	218.85	9.72	2.66	0.11	9.72	2.66

Main Results for each time segment

Main results: (07:45-08:00)

Arm	Total Demand (Veh/hr)	Junction Arrivals (Veh)	Entry Flow (Veh/hr)	Exit Flow (Veh/hr)	Circulating Flow (Veh/hr)	Pedestrian Demand (Ped/hr)	Capacity (Veh/hr)	Saturation Capacity (Veh/hr)	RFC	Start Queue (Veh)	End Queue (Veh)	Delay (s)	LOS
1	207.79	51.95	207.35	258.28	73.57	0.00	2119.24	1928.90	0.098	0.00	0.11	1.882	A
2	149.06	37.27	148.68	185.53	95.40	0.00	1715.86	1270.33	0.087	0.00	0.09	2.297	A
3	90.34	22.59	90.08	80.37	163.71	0.00	1441.12	694.92	0.063	0.00	0.07	2.864	A
4	119.70	29.93	119.37	41.31	212.48	0.00	1531.66	537.02	0.078	0.00	0.08	2.549	A

Main results: (08:00-08:15)

Arm	Total Demand (Veh/hr)	Junction Arrivals (Veh)	Entry Flow (Veh/hr)	Exit Flow (Veh/hr)	Circulating Flow (Veh/hr)	Pedestrian Demand (Ped/hr)	Capacity (Veh/hr)	Saturation Capacity (Veh/hr)	RFC	Start Queue (Veh)	End Queue (Veh)	Delay (s)	LOS
1	248.12	62.03	248.02	309.09	88.05	0.00	2108.73	1928.90	0.118	0.11	0.13	1.934	A
2	178.00	44.50	177.91	221.95	114.12	0.00	1703.92	1270.33	0.104	0.09	0.12	2.358	A
3	107.88	26.97	107.82	96.15	195.88	0.00	1421.92	694.92	0.076	0.07	0.08	2.739	A
4	142.94	35.73	142.86	49.42	254.26	0.00	1504.61	537.02	0.095	0.08	0.10	2.643	A

Main results: (08:15-08:30)

Arm	Total Demand (Veh/hr)	Junction Arrivals (Veh)	Entry Flow (Veh/hr)	Exit Flow (Veh/hr)	Circulating Flow (Veh/hr)	Pedestrian Demand (Ped/hr)	Capacity (Veh/hr)	Saturation Capacity (Veh/hr)	RFC	Start Queue (Veh)	End Queue (Veh)	Delay (s)	LOS
1	303.88	75.97	303.74	378.51	107.83	0.00	2094.37	1928.89	0.145	0.13	0.17	2.010	A
2	218.00	54.50	217.87	271.80	139.76	0.00	1687.58	1270.33	0.129	0.12	0.15	2.449	A
3	132.12	33.03	132.03	117.75	239.89	0.00	1395.68	694.92	0.095	0.08	0.10	2.848	A
4	175.06	43.77	174.94	60.52	311.40	0.00	1467.65	537.02	0.119	0.10	0.13	2.784	A

Main results: (08:30-08:45)

Arm	Total Demand (Veh/hr)	Junction Arrivals (Veh)	Entry Flow (Veh/hr)	Exit Flow (Veh/hr)	Circulating Flow (Veh/hr)	Pedestrian Demand (Ped/hr)	Capacity (Veh/hr)	Saturation Capacity (Veh/hr)	RFC	Start Queue (Veh)	End Queue (Veh)	Delay (s)	LOS
1	303.88	75.97	303.88	378.75	107.90	0.00	2094.32	1928.89	0.145	0.17	0.17	2.010	A
2	218.00	54.50	218.00	271.95	139.83	0.00	1687.53	1270.33	0.129	0.15	0.15	2.449	A
3	132.12	33.03	132.12	117.81	240.02	0.00	1395.60	694.92	0.095	0.10	0.10	2.848	A
4	175.06	43.77	175.06	60.56	311.59	0.00	1467.52	537.02	0.119	0.13	0.14	2.784	A

Main results: (08:45-09:00)

Arm	Total Demand (Veh/hr)	Junction Arrivals (Veh)	Entry Flow (Veh/hr)	Exit Flow (Veh/hr)	Circulating Flow (Veh/hr)	Pedestrian Demand (Ped/hr)	Capacity (Veh/hr)	Saturation Capacity (Veh/hr)	RFC	Start Queue (Veh)	End Queue (Veh)	Delay (s)	LOS
1	248.12	62.03	248.26	309.48	88.17	0.00	2108.64	1928.90	0.118	0.17	0.13	1.934	A
2	178.00	44.50	178.12	222.19	114.24	0.00	1703.84	1270.33	0.104	0.15	0.12	2.359	A
3	107.88	26.97	107.97	96.25	196.11	0.00	1421.79	694.92	0.076	0.10	0.08	2.741	A
4	142.94	35.73	143.06	49.48	254.60	0.00	1504.40	537.02	0.095	0.14	0.11	2.646	A

Main results: (09:00-09:15)

Arm	Total Demand (Veh/hr)	Junction Arrivals (Veh)	Entry Flow (Veh/hr)	Exit Flow (Veh/hr)	Circulating Flow (Veh/hr)	Pedestrian Demand (Ped/hr)	Capacity (Veh/hr)	Saturation Capacity (Veh/hr)	RFC	Start Queue (Veh)	End Queue (Veh)	Delay (s)	LOS
1	207.79	51.95	207.89	259.14	73.83	0.00	2119.05	1928.90	0.098	0.13	0.11	1.882	A
2	149.06	37.27	149.15	186.05	95.66	0.00	1715.69	1270.33	0.087	0.12	0.10	2.297	A
3	90.34	22.59	90.40	80.60	164.21	0.00	1440.82	694.92	0.063	0.08	0.07	2.667	A
4	119.70	29.93	119.78	41.43	213.19	0.00	1531.20	537.02	0.078	0.11	0.09	2.550	A

Queueing Delay Results for each time segment

Queueing Delay results: (07:45-08:00)

Arm	Queueing Total Delay (Veh-min)	Queueing Rate Of Delay (Veh-min/min)	Average Delay Per Arriving Vehicle (s)	Unsignalised Level Of Service	Signalised Level Of Service
1	1.61	0.11	1.882	A	A
2	1.40	0.09	2.297	A	A
3	0.99	0.07	2.864	A	A
4	1.25	0.08	2.549	A	A

Queueing Delay results: (08:00-08:15)

Arm	Queueing Total Delay (Veh-min)	Queueing Rate Of Delay (Veh-min/min)	Average Delay Per Arriving Vehicle (s)	Unsignalised Level Of Service	Signalised Level Of Service
1	1.98	0.13	1.934	A	A
2	1.73	0.12	2.358	A	A
3	1.21	0.08	2.739	A	A
4	1.55	0.10	2.643	A	A

Queueing Delay results: (08:15-08:30)

Arm	Queueing Total Delay (Veh-min)	Queueing Rate Of Delay (Veh-min/min)	Average Delay Per Arriving Vehicle (s)	Unsignalised Level Of Service	Signalised Level Of Service
1	2.52	0.17	2.010	A	A
2	2.20	0.15	2.449	A	A
3	1.55	0.10	2.848	A	A
4	2.00	0.13	2.784	A	A

Queueing Delay results: (08:30-08:45)

Arm	Queueing Total Delay (Veh-min)	Queueing Rate Of Delay (Veh-min/min)	Average Delay Per Arriving Vehicle (s)	Unsignalised Level Of Service	Signalised Level Of Service
1	2.54	0.17	2.010	A	A
2	2.22	0.15	2.449	A	A
3	1.56	0.10	2.848	A	A
4	2.03	0.14	2.784	A	A

Queueing Delay results: (08:45-09:00)

Arm	Queueing Total Delay (Veh-min)	Queueing Rate Of Delay (Veh-min/min)	Average Delay Per Arriving Vehicle (s)	Unsignalised Level Of Service	Signalised Level Of Service
1	2.02	0.13	1.934	A	A
2	1.77	0.12	2.359	A	A
3	1.25	0.08	2.741	A	A
4	1.60	0.11	2.646	A	A

Queueing Delay results: (09:00-09:15)

Arm	Queueing Total Delay (Veh-min)	Queueing Rate Of Delay (Veh-min/min)	Average Delay Per Arriving Vehicle (s)	Unsignalised Level Of Service	Signalised Level Of Service
1	1.65	0.11	1.882	A	A
2	1.44	0.10	2.297	A	A
3	1.02	0.07	2.667	A	A
4	1.29	0.09	2.550	A	A

Queue Variation Results for each time segment

Queue Variation results: (07:45-08:00)

Arm	Mean (Veh)	Q05 (Veh)	Q50 (Veh)	Q90 (Veh)	Q95 (Veh)	Percentile Message	Marker Message	Probability Of Reaching Or Exceeding Marker	Probability Of Exactly Reaching Marker
1	0.11	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
2	0.09	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
3	0.07	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
4	0.08	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A

Queue Variation results: (08:00-08:15)

Arm	Mean (Veh)	Q05 (Veh)	Q50 (Veh)	Q90 (Veh)	Q95 (Veh)	Percentile Message	Marker Message	Probability Of Reaching Or Exceeding Marker	Probability Of Exactly Reaching Marker
1	0.13	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
2	0.12	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
3	0.08	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
4	0.10	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A

Queue Variation results: (08:15-08:30)

Arm	Mean (Veh)	Q05 (Veh)	Q50 (Veh)	Q90 (Veh)	Q95 (Veh)	Percentile Message	Marker Message	Probability Of Reaching Or Exceeding Marker	Probability Of Exactly Reaching Marker
1	0.17	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
2	0.15	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
3	0.10	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
4	0.13	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A

Queue Variation results: (08:30-08:45)

Arm	Mean (Veh)	Q05 (Veh)	Q50 (Veh)	Q90 (Veh)	Q95 (Veh)	Percentile Message	Marker Message	Probability Of Reaching Or Exceeding Marker	Probability Of Exactly Reaching Marker
1	0.17	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
2	0.15	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
3	0.10	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
4	0.14	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A

Queue Variation results: (08:45-09:00)

Arm	Mean (Veh)	Q05 (Veh)	Q50 (Veh)	Q90 (Veh)	Q95 (Veh)	Percentile Message	Marker Message	Probability Of Reaching Or Exceeding Marker	Probability Of Exactly Reaching Marker
1	0.13	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
2	0.12	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
3	0.08	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
4	0.11	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A

Queue Variation results: (09:00-09:15)

Arm	Mean (Veh)	Q05 (Veh)	Q50 (Veh)	Q90 (Veh)	Q95 (Veh)	Percentile Message	Marker Message	Probability Of Reaching Or Exceeding Marker	Probability Of Exactly Reaching Marker
1	0.11	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
2	0.10	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
3	0.07	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
4	0.09	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A

Existing Layout - Base Year 2013, PM

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Geometry	Arm 4 - Roundabout Geometry	Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution.

Analysis Set Details

Name	Roundabout Capacity Model	Description	Include In Report	Use Specific Demand Set(s)	Specific Demand Set(s)	Locked	Network Flow Scaling Factor (%)	Network Capacity Scaling Factor (%)	Reason For Scaling Factors
Existing Layout	ARCADY		✓				100.000	100.000	

Demand Set Details

Name	Scenario Name	Time Period Name	Description	Traffic Profile Type	Model Start Time (HH:mm)	Model Finish Time (HH:mm)	Model Time Period Length (min)	Time Segment Length (min)	Results For Central Hour Only	Single Time Segment Only	Locked	Run Automatically	Use Relationship	Relationship
Base Year 2013, PM	Base Year 2013	PM		ONE HOUR	16:45	18:15	90	15				✓		

Junction Network

Junctions

Name	Junction Type	Arm Order	Grade Separated	Large Roundabout	Do Geometric Delay	Junction Delay (s)	Junction LOS
(untitled)	Roundabout	1,2,3,4				2.48	A

Junction Network Options

Driving Side	Lighting
Left	Normal/unknown

Arms

Arms

Arm	Name	Description
1	Exeter Road (North)	
2	Exeter Road (South)	
3	Bowhill	
4	Swallow Way	

Capacity Options

Arm	Minimum Capacity (PCU/hr)	Maximum Capacity (PCU/hr)	Assume Flat Start Profile	Initial Queue (PCU)
1	0.00	99999.00		0.00
2	0.00	99999.00		0.00
3	0.00	99999.00		0.00
4	0.00	99999.00		0.00

Roundabout Geometry

Arm	V - Approach road half-width (m)	E - Entry width (m)	F - Effective flare length (m)	R - Entry radius (m)	D - Inscribed circle diameter (m)	PHI - Conflict (entry) angle (deg)	Exit Only
1	7.00	8.00	30.00	37.50	30.00	60.00	
2	4.50	8.00	20.00	12.50	30.00	50.00	
3	3.00	8.00	25.00	10.00	30.00	60.00	
4	3.00	7.00	35.00	62.50	30.00	60.00	

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Pedestrian Crossings

Arm	Crossing Type
1	None
2	None
3	None
4	None

Slope / Intercept / Capacity

Roundabout Slope and Intercept used in model

Arm	Enter slope and intercept directly	Entered slope	Entered Intercept (PCU/hr)	Final Slope	Final Intercept (PCU/hr)
1		(calculated)	(calculated)	0.735	2200.148
2		(calculated)	(calculated)	0.656	1841.551
3		(calculated)	(calculated)	0.580	1552.365
4		(calculated)	(calculated)	0.630	1669.173

The slope and intercept shown above include any corrections and adjustments.

Traffic Flows

Demand Set Data Options

Default Vehicle Mix	Vehicle Mix Varies Over Time	Vehicle Mix Varies Over Turn	Vehicle Mix Varies Over Entry	Vehicle Mix Source	PCU Factor for a HV (PCU)	Default Turning Proportions	Estimate from entry/exit counts	Turning Proportions Vary Over Time	Turning Proportions Vary Over Turn	Turning Proportions Vary Over Entry
		✓	✓	HV Percentages	2.00				✓	✓

Entry Flows

General Flows Data

Arm	Profile Type	Use Turning Counts	Average Demand Flow (Veh/hr)	Flow Scaling Factor (%)
1	ONE HOUR	✓	347.00	100.000
2	ONE HOUR	✓	315.00	100.000
3	ONE HOUR	✓	130.00	100.000
4	ONE HOUR	✓	90.00	100.000

Turning Proportions

Turning Counts or Proportions (Veh/hr) - Junction 1 (for whole period)

		To			
		1	2	3	4
From	1	0.000	165.000	119.000	63.000
	2	245.000	0.000	3.000	67.000
	3	100.000	4.000	0.000	26.000
	4	46.000	25.000	19.000	0.000

Turning Proportions (Veh) - Junction 1 (for whole period)

		To			
		1	2	3	4
From	1	0.00	0.48	0.34	0.18
	2	0.78	0.00	0.01	0.21
	3	0.77	0.03	0.00	0.20
	4	0.51	0.28	0.21	0.00

Vehicle Mix

Average PCU Per Vehicle - Junction 1 (for whole period)

		To			
		1	2	3	4
From	1	1.000	1.000	1.000	1.000
	2	1.000	1.000	1.000	1.000
	3	1.000	1.000	1.000	1.000
	4	1.000	1.000	1.000	1.000

Heavy Vehicle Percentages - Junction 1 (for whole period)

		To			
		1	2	3	4
From	1	0.000	0.000	0.000	0.000
	2	0.000	0.000	0.000	0.000
	3	0.000	0.000	0.000	0.000
	4	0.000	0.000	0.000	0.000

Results

Results Summary for whole modelled period

Arm	Max RFC	Max Delay (s)	Max Queue (Veh)	Max LOS	Average Demand (Veh/hr)	Total Junction Arrivals (Veh)	Total Queueing Delay (Veh-min)	Average Queueing Delay (s)	Rate Of Queueing Delay (Veh-min/min)	Inclusive Total Queueing Delay (Veh-min)	Inclusive Average Queueing Delay (s)
1	0.18	2.02	0.21	A	318.41	477.62	15.53	1.95	0.17	15.53	1.95
2	0.20	2.67	0.26	A	289.05	433.57	18.26	2.53	0.20	18.26	2.53
3	0.11	3.08	0.12	A	119.29	178.94	8.74	2.93	0.10	8.74	2.93
4	0.07	2.71	0.07	A	82.59	123.88	5.38	2.61	0.06	5.38	2.61

Main Results for each time segment

Main results: (16:45-17:00)

Arm	Total Demand (Veh/hr)	Junction Arrivals (Veh)	Entry Flow (Veh/hr)	Exit Flow (Veh/hr)	Circulating Flow (Veh/hr)	Pedestrian Demand (Ped/hr)	Capacity (Veh/hr)	Saturation Capacity (Veh/hr)	RFC	Start Queue (Veh)	End Queue (Veh)	Delay (s)	LOS
1	261.24	65.31	260.69	293.55	36.04	0.00	2173.66	1906.01	0.120	0.00	0.14	1.881	A
2	237.15	59.29	236.52	145.73	151.00	0.00	1742.46	1078.49	0.136	0.00	0.16	2.389	A
3	97.87	24.47	97.57	105.92	281.60	0.00	1388.97	731.74	0.070	0.00	0.08	2.787	A
4	67.76	16.94	67.57	117.15	262.02	0.00	1504.20	772.45	0.045	0.00	0.05	2.505	A

Main results: (17:00-17:15)

Arm	Total Demand (Veh/hr)	Junction Arrivals (Veh)	Entry Flow (Veh/hr)	Exit Flow (Veh/hr)	Circulating Flow (Veh/hr)	Pedestrian Demand (Ped/hr)	Capacity (Veh/hr)	Saturation Capacity (Veh/hr)	RFC	Start Queue (Veh)	End Queue (Veh)	Delay (s)	LOS
1	311.95	77.99	311.82	351.30	43.13	0.00	2168.45	1906.00	0.144	0.14	0.17	1.938	A
2	283.18	70.79	283.02	174.33	180.62	0.00	1723.02	1078.48	0.164	0.16	0.20	2.499	A
3	116.87	29.22	116.79	126.70	336.94	0.00	1356.86	731.74	0.086	0.08	0.09	2.902	A
4	80.91	20.23	80.88	140.17	313.56	0.00	1471.74	772.45	0.055	0.05	0.06	2.587	A

Main results: (17:15-17:30)

Arm	Total Demand (Veh/hr)	Junction Arrivals (Veh)	Entry Flow (Veh/hr)	Exit Flow (Veh/hr)	Circulating Flow (Veh/hr)	Pedestrian Demand (Ped/hr)	Capacity (Veh/hr)	Saturation Capacity (Veh/hr)	RFC	Start Queue (Veh)	End Queue (Veh)	Delay (s)	LOS
1	382.05	95.51	381.87	430.19	52.81	0.00	2161.33	1906.00	0.177	0.17	0.21	2.023	A
2	346.82	86.71	346.58	213.49	221.19	0.00	1698.39	1078.49	0.204	0.20	0.26	2.686	A
3	143.13	35.78	143.02	155.16	412.61	0.00	1312.95	731.74	0.109	0.09	0.12	3.076	A
4	99.09	24.77	99.03	171.65	363.98	0.00	1427.41	772.45	0.069	0.06	0.07	2.709	A

Main results: (17:30-17:45)

Arm	Total Demand (Veh/hr)	Junction Arrivals (Veh)	Entry Flow (Veh/hr)	Exit Flow (Veh/hr)	Circulating Flow (Veh/hr)	Pedestrian Demand (Ped/hr)	Capacity (Veh/hr)	Saturation Capacity (Veh/hr)	RFC	Start Queue (Veh)	End Queue (Veh)	Delay (s)	LOS
1	382.05	95.51	382.05	430.50	52.85	0.00	2161.30	1906.00	0.177	0.21	0.21	2.023	A
2	346.82	86.71	346.82	213.60	221.30	0.00	1698.32	1078.49	0.204	0.26	0.26	2.687	A
3	143.13	35.78	143.13	155.24	412.88	0.00	1312.79	731.74	0.109	0.12	0.12	3.077	A
4	99.09	24.77	99.09	171.76	384.25	0.00	1427.24	772.45	0.069	0.07	0.07	2.709	A

Main results: (17:45-18:00)

Arm	Total Demand (Veh/hr)	Junction Arrivals (Veh)	Entry Flow (Veh/hr)	Exit Flow (Veh/hr)	Circulating Flow (Veh/hr)	Pedestrian Demand (Ped/hr)	Capacity (Veh/hr)	Saturation Capacity (Veh/hr)	RFC	Start Queue (Veh)	End Queue (Veh)	Delay (s)	LOS
1	311.95	77.99	312.13	351.80	43.19	0.00	2168.40	1906.00	0.144	0.21	0.17	1.940	A
2	283.18	70.79	283.42	174.51	180.80	0.00	1722.90	1078.48	0.164	0.26	0.20	2.500	A
3	116.87	29.22	116.98	126.84	337.39	0.00	1356.60	731.74	0.086	0.12	0.09	2.903	A
4	80.91	20.23	80.97	140.35	314.02	0.00	1471.46	772.45	0.055	0.07	0.06	2.588	A

Main results: (18:00-18:15)

Arm	Total Demand (Veh/hr)	Junction Arrivals (Veh)	Entry Flow (Veh/hr)	Exit Flow (Veh/hr)	Circulating Flow (Veh/hr)	Pedestrian Demand (Ped/hr)	Capacity (Veh/hr)	Saturation Capacity (Veh/hr)	RFC	Start Queue (Veh)	End Queue (Veh)	Delay (s)	LOS
1	261.24	65.31	261.37	294.57	36.16	0.00	2173.57	1906.01	0.120	0.17	0.14	1.884	A
2	237.15	59.29	237.31	146.13	151.40	0.00	1742.20	1078.49	0.136	0.20	0.16	2.393	A
3	97.87	24.47	97.94	106.21	282.50	0.00	1388.45	731.74	0.070	0.09	0.08	2.791	A
4	67.76	16.94	67.80	117.52	262.93	0.00	1503.63	772.45	0.045	0.06	0.05	2.508	A

Queueing Delay Results for each time segment

Queueing Delay results: (16:45-17:00)

Arm	Queueing Total Delay (Veh-min)	Queueing Rate Of Delay (Veh-min/min)	Average Delay Per Arriving Vehicle (s)	Unsignalised Level Of Service	Signalised Level Of Service
1	2.02	0.13	1.881	A	A
2	2.32	0.15	2.389	A	A
3	1.12	0.07	2.787	A	A
4	0.70	0.05	2.505	A	A

Queueing Delay results: (17:00-17:15)

Arm	Queueing Total Delay (Veh-min)	Queueing Rate Of Delay (Veh-min/min)	Average Delay Per Arriving Vehicle (s)	Unsignalised Level Of Service	Signalised Level Of Service
1	2.49	0.17	1.938	A	A
2	2.91	0.19	2.499	A	A
3	1.39	0.09	2.902	A	A
4	0.86	0.06	2.587	A	A

Queueing Delay results: (17:15-17:30)

Arm	Queueing Total Delay (Veh-min)	Queueing Rate Of Delay (Veh-min/min)	Average Delay Per Arriving Vehicle (s)	Unsignalised Level Of Service	Signalised Level Of Service
1	3.18	0.21	2.023	A	A
2	3.80	0.25	2.666	A	A
3	1.81	0.12	3.076	A	A
4	1.10	0.07	2.709	A	A

Queueing Delay results: (17:30-17:45)

Arm	Queueing Total Delay (Veh-min)	Queueing Rate Of Delay (Veh-min/min)	Average Delay Per Arriving Vehicle (s)	Unsignalised Level Of Service	Signalised Level Of Service
1	3.21	0.21	2.023	A	A
2	3.85	0.26	2.667	A	A
3	1.83	0.12	3.077	A	A
4	1.12	0.07	2.709	A	A

Queueing Delay results: (17:45-18:00)

Arm	Queueing Total Delay (Veh-min)	Queueing Rate Of Delay (Veh-min/min)	Average Delay Per Arriving Vehicle (s)	Unsignalised Level Of Service	Signalised Level Of Service
1	2.55	0.17	1.940	A	A
2	2.99	0.20	2.500	A	A
3	1.44	0.10	2.903	A	A
4	0.88	0.06	2.588	A	A

Queueing Delay results: (18:00-18:15)

Arm	Queueing Total Delay (Veh-min)	Queueing Rate Of Delay (Veh-min/min)	Average Delay Per Arriving Vehicle (s)	Unsignalised Level Of Service	Signalised Level Of Service
1	2.07	0.14	1.884	A	A
2	2.40	0.16	2.393	A	A
3	1.15	0.08	2.791	A	A
4	0.72	0.05	2.508	A	A

Queue Variation Results for each time segment

Queue Variation results: (16:45-17:00)

Arm	Mean (Veh)	Q05 (Veh)	Q50 (Veh)	Q90 (Veh)	Q95 (Veh)	Percentile Message	Marker Message	Probability Of Reaching Or Exceeding Marker	Probability Of Exactly Reaching Marker
1	0.14	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
2	0.16	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
3	0.08	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
4	0.05	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A

Queue Variation results: (17:00-17:15)

Arm	Mean (Veh)	Q05 (Veh)	Q50 (Veh)	Q90 (Veh)	Q95 (Veh)	Percentile Message	Marker Message	Probability Of Reaching Or Exceeding Marker	Probability Of Exactly Reaching Marker
1	0.17	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
2	0.20	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
3	0.09	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
4	0.06	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A

Queue Variation results: (17:15-17:30)

Arm	Mean (Veh)	Q05 (Veh)	Q50 (Veh)	Q90 (Veh)	Q95 (Veh)	Percentile Message	Marker Message	Probability Of Reaching Or Exceeding Marker	Probability Of Exactly Reaching Marker
1	0.21	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
2	0.26	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
3	0.12	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
4	0.07	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A

Queue Variation results: (17:30-17:45)

Arm	Mean (Veh)	Q05 (Veh)	Q50 (Veh)	Q90 (Veh)	Q95 (Veh)	Percentile Message	Marker Message	Probability Of Reaching Or Exceeding Marker	Probability Of Exactly Reaching Marker
1	0.21	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
2	0.26	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
3	0.12	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
4	0.07	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A

Queue Variation results: (17:45-18:00)

Arm	Mean (Veh)	Q05 (Veh)	Q50 (Veh)	Q90 (Veh)	Q95 (Veh)	Percentile Message	Marker Message	Probability Of Reaching Or Exceeding Marker	Probability Of Exactly Reaching Marker
1	0.17	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
2	0.20	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
3	0.09	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
4	0.06	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A

Queue Variation results: (18:00-18:15)

Arm	Mean (Veh)	Q05 (Veh)	Q50 (Veh)	Q90 (Veh)	Q95 (Veh)	Percentile Message	Marker Message	Probability Of Reaching Or Exceeding Marker	Probability Of Exactly Reaching Marker
1	0.14	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
2	0.16	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
3	0.08	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
4	0.05	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A

Existing Layout - Testing Year 2023 + Dev, AM Peak

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Geometry	Arm 4 - Roundabout Geometry	Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution.

Analysis Set Details

Name	Roundabout Capacity Model	Description	Include In Report	Use Specific Demand Set(s)	Specific Demand Set(s)	Locked	Network Flow Scaling Factor (%)	Network Capacity Scaling Factor (%)	Reason For Scaling Factors
Existing Layout	ARCADY		✓				100.000	100.000	

Demand Set Details

Name	Scenario Name	Time Period Name	Description	Traffic Profile Type	Model Start Time (HH:mm)	Model Finish Time (HH:mm)	Model Time Period Length (min)	Time Segment Length (min)	Results For Central Hour Only	Single Time Segment Only	Locked	Run Automatically	Use Relationship	Relationship
Testing Year 2023 + Dev, AM Peak	Testing Year 2023 + Dev	AM Peak		ONE HOUR	07:45	09:15	90	15				✓		

Junction Network

Junctions

Name	Junction Type	Arm Order	Grade Separated	Large Roundabout	Do Geometric Delay	Junction Delay (s)	Junction LOS
(untitled)	Roundabout	1,2,3,4				2.91	A

Junction Network Options

Driving Side	Lighting
Left	Normal/unknown

Arms

Arms

Arm	Name	Description
1	Exeter Road (North)	
2	Exeter Road (South)	
3	Bowhill	
4	Swallow Way	

Capacity Options

Arm	Minimum Capacity (PCU/hr)	Maximum Capacity (PCU/hr)	Assume Flat Start Profile	Initial Queue (PCU)
1	0.00	99999.00		0.00
2	0.00	99999.00		0.00
3	0.00	99999.00		0.00
4	0.00	99999.00		0.00

Roundabout Geometry

Arm	V - Approach road half-width (m)	E - Entry width (m)	F - Effective flare length (m)	R - Entry radius (m)	D - Inscribed circle diameter (m)	PHI - Conflict (entry) angle (deg)	Exit Only
1	7.00	8.00	30.00	37.50	30.00	60.00	
2	4.50	8.00	20.00	12.50	30.00	50.00	
3	3.00	8.00	25.00	10.00	30.00	60.00	
4	3.00	7.00	35.00	62.50	30.00	60.00	

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Pedestrian Crossings

Arm	Crossing Type
1	None
2	None
3	None
4	None

Slope / Intercept / Capacity

Roundabout Slope and Intercept used in model

Arm	Enter slope and intercept directly	Entered slope	Entered intercept (PCU/hr)	Final Slope	Final Intercept (PCU/hr)
1		(calculated)	(calculated)	0.735	2200.146
2		(calculated)	(calculated)	0.656	1841.551
3		(calculated)	(calculated)	0.580	1552.365
4		(calculated)	(calculated)	0.630	1669.173

The slope and intercept shown above include any corrections and adjustments.

Traffic Flows

Demand Set Data Options

Default Vehicle Mix	Vehicle Mix Varies Over Time	Vehicle Mix Varies Over Turn	Vehicle Mix Varies Over Entry	Vehicle Mix Source	PCU Factor for a HV (PCU)	Default Turning Proportions	Estimate from entry/exit counts	Turning Proportions Vary Over Time	Turning Proportions Vary Over Turn	Turning Proportions Vary Over Entry
		✓	✓	HV Percentages	2.00				✓	✓

Entry Flows

General Flows Data

Arm	Profile Type	Use Turning Counts	Average Demand Flow (Veh/hr)	Flow Scaling Factor (%)
1	ONE HOUR	✓	312.00	100.000
2	ONE HOUR	✓	296.00	100.000
3	ONE HOUR	✓	137.00	100.000
4	ONE HOUR	✓	392.00	100.000

Turning Proportions

Turning Counts or Proportions (Veh/hr) - Junction 1 (for whole period)

		To			
		1	2	3	4
From	1	0.000	198.000	82.000	32.000
	2	198.000	0.000	9.000	69.000
	3	120.000	2.000	0.000	15.000
	4	71.000	290.000	31.000	0.000

Turning Proportions (Veh) - Junction 1 (for whole period)

		To			
		1	2	3	4
From	1	0.00	0.63	0.26	0.10
	2	0.67	0.00	0.03	0.30
	3	0.88	0.01	0.00	0.11
	4	0.18	0.74	0.08	0.00

Vehicle Mix

Average PCU Per Vehicle - Junction 1 (for whole period)

		To			
		1	2	3	4
From	1	1.000	1.015	1.000	1.031
	2	1.039	1.000	1.000	1.027
	3	1.010	1.000	1.000	1.000
	4	1.000	1.000	1.000	1.000

Heavy Vehicle Percentages - Junction 1 (for whole period)

		To			
		1	2	3	4
From	1	0.000	1.500	0.000	3.100
	2	3.900	0.000	0.000	2.700
	3	1.000	0.000	0.000	0.000
	4	0.000	0.000	0.000	0.000

Results

Results Summary for whole modelled period

Arm	Max RFC	Max Delay (s)	Max Queue (Veh)	Max LOS	Average Demand (Veh/hr)	Total Junction Arrivals (Veh)	Total Queuing Delay (Veh-min)	Average Queuing Delay (s)	Rate Of Queuing Delay (Veh-min/min)	Inclusive Total Queuing Delay (Veh-min)	Inclusive Average Queuing Delay (s)
1	0.18	2.29	0.22	A	286.30	429.44	15.51	2.17	0.17	15.51	2.17
2	0.19	2.66	0.24	A	271.61	407.42	17.23	2.54	0.19	17.23	2.54
3	0.11	3.05	0.13	A	125.71	188.57	9.16	2.91	0.10	9.16	2.91
4	0.30	3.56	0.43	A	359.71	539.56	29.27	3.25	0.33	29.27	3.25

Main Results for each time segment

Main results: (07:45-08:00)

Arm	Total Demand (Veh/hr)	Junction Arrivals (Veh)	Entry Flow (Veh/hr)	Exit Flow (Veh/hr)	Circulating Flow (Veh/hr)	Pedestrian Demand (Ped/hr)	Capacity (Veh/hr)	Saturation Capacity (Veh/hr)	RFC	Start Queue (Veh)	End Queue (Veh)	Delay (s)	LOS
1	234.89	58.72	234.36	292.01	242.38	0.00	1996.63	1743.65	0.118	0.00	0.13	2.043	A
2	222.84	55.71	222.25	367.84	108.89	0.00	1711.07	1337.23	0.130	0.00	0.15	2.416	A
3	103.14	25.79	102.62	91.61	239.53	0.00	1398.30	660.71	0.074	0.00	0.08	2.783	A
4	295.12	73.78	294.15	102.12	240.23	0.00	1513.70	709.90	0.195	0.00	0.24	2.947	A

Main results: (08:00-08:15)

Arm	Total Demand (Veh/hr)	Junction Arrivals (Veh)	Entry Flow (Veh/hr)	Exit Flow (Veh/hr)	Circulating Flow (Veh/hr)	Pedestrian Demand (Ped/hr)	Capacity (Veh/hr)	Saturation Capacity (Veh/hr)	RFC	Start Queue (Veh)	End Queue (Veh)	Delay (s)	LOS
1	280.48	70.12	280.35	349.49	290.14	0.00	1961.96	1743.64	0.143	0.13	0.17	2.140	A
2	266.10	66.52	265.95	440.21	130.28	0.00	1697.40	1337.23	0.157	0.15	0.19	2.514	A
3	123.16	30.79	123.08	109.61	266.62	0.00	1368.27	660.71	0.090	0.08	0.10	2.890	A
4	352.40	88.10	352.12	122.20	287.51	0.00	1483.10	709.90	0.238	0.24	0.31	3.183	A

Main results: (08:15-08:30)

Arm	Total Demand (Veh/hr)	Junction Arrivals (Veh)	Entry Flow (Veh/hr)	Exit Flow (Veh/hr)	Circulating Flow (Veh/hr)	Pedestrian Demand (Ped/hr)	Capacity (Veh/hr)	Saturation Capacity (Veh/hr)	RFC	Start Queue (Veh)	End Queue (Veh)	Delay (s)	LOS
1	343.52	85.88	343.31	427.97	355.25	0.00	1914.70	1743.64	0.179	0.17	0.22	2.290	A
2	325.90	81.48	325.68	539.03	159.54	0.00	1678.71	1337.23	0.194	0.19	0.24	2.660	A
3	150.84	37.71	150.72	134.23	350.99	0.00	1329.95	660.71	0.113	0.10	0.13	3.052	A
4	431.60	107.90	431.14	149.64	352.08	0.00	1441.31	709.90	0.299	0.31	0.43	3.561	A

Main results: (08:30-08:45)

Arm	Total Demand (Veh/hr)	Junction Arrivals (Veh)	Entry Flow (Veh/hr)	Exit Flow (Veh/hr)	Circulating Flow (Veh/hr)	Pedestrian Demand (Ped/hr)	Capacity (Veh/hr)	Saturation Capacity (Veh/hr)	RFC	Start Queue (Veh)	End Queue (Veh)	Delay (s)	LOS
1	343.52	85.88	343.52	428.29	355.63	0.00	1914.43	1743.64	0.179	0.22	0.22	2.291	A
2	325.90	81.48	325.90	539.50	159.65	0.00	1678.64	1337.23	0.194	0.24	0.24	2.660	A
3	150.84	37.71	150.84	134.32	351.22	0.00	1329.82	660.71	0.113	0.13	0.13	3.052	A
4	431.60	107.90	431.60	149.74	352.32	0.00	1441.15	709.90	0.299	0.43	0.43	3.565	A

Main results: (08:45-09:00)

Arm	Total Demand (Veh/hr)	Junction Arrivals (Veh)	Entry Flow (Veh/hr)	Exit Flow (Veh/hr)	Circulating Flow (Veh/hr)	Pedestrian Demand (Ped/hr)	Capacity (Veh/hr)	Saturation Capacity (Veh/hr)	RFC	Start Queue (Veh)	End Queue (Veh)	Delay (s)	LOS
1	280.48	70.12	280.69	350.03	290.74	0.00	1961.52	1743.64	0.143	0.22	0.17	2.143	A
2	266.10	66.52	266.31	440.97	130.46	0.00	1697.29	1337.23	0.157	0.24	0.19	2.517	A
3	123.16	30.79	123.27	109.77	287.00	0.00	1368.04	660.71	0.090	0.13	0.10	2.891	A
4	352.40	88.10	352.85	122.38	287.92	0.00	1482.84	709.90	0.238	0.43	0.31	3.186	A

Main results: (09:00-09:15)

Arm	Total Demand (Veh/hr)	Junction Arrivals (Veh)	Entry Flow (Veh/hr)	Exit Flow (Veh/hr)	Circulating Flow (Veh/hr)	Pedestrian Demand (Ped/hr)	Capacity (Veh/hr)	Saturation Capacity (Veh/hr)	RFC	Start Queue (Veh)	End Queue (Veh)	Delay (s)	LOS
1	234.89	58.72	235.02	293.08	243.40	0.00	1995.89	1743.65	0.118	0.17	0.13	2.044	A
2	222.84	55.71	222.99	369.19	109.23	0.00	1710.85	1337.23	0.130	0.19	0.15	2.419	A
3	103.14	25.79	103.22	91.91	240.31	0.00	1395.84	660.71	0.074	0.10	0.08	2.784	A
4	295.12	73.78	295.40	102.45	241.08	0.00	1513.15	709.90	0.195	0.31	0.24	2.958	A

Queueing Delay Results for each time segment

Queueing Delay results: (07:45-08:00)

Arm	Queueing Total Delay (Veh-min)	Queueing Rate Of Delay (Veh-min/min)	Average Delay Per Arriving Vehicle (s)	Unsignalised Level Of Service	Signalised Level Of Service
1	1.97	0.13	2.043	A	A
2	2.21	0.15	2.416	A	A
3	1.17	0.08	2.783	A	A
4	3.55	0.24	2.947	A	A

Queueing Delay results: (08:00-08:15)

Arm	Queueing Total Delay (Veh-min)	Queueing Rate Of Delay (Veh-min/min)	Average Delay Per Arriving Vehicle (s)	Unsignalised Level Of Service	Signalised Level Of Service
1	2.47	0.16	2.140	A	A
2	2.75	0.18	2.514	A	A
3	1.46	0.10	2.890	A	A
4	4.59	0.31	3.183	A	A

Queueing Delay results: (08:15-08:30)

Arm	Queueing Total Delay (Veh-min)	Queueing Rate Of Delay (Veh-min/min)	Average Delay Per Arriving Vehicle (s)	Unsignalised Level Of Service	Signalised Level Of Service
1	3.24	0.22	2.290	A	A
2	3.56	0.24	2.660	A	A
3	1.89	0.13	3.052	A	A
4	6.27	0.42	3.561	A	A

Queueing Delay results: (08:30-08:45)

Arm	Queueing Total Delay (Veh-min)	Queueing Rate Of Delay (Veh-min/min)	Average Delay Per Arriving Vehicle (s)	Unsignalised Level Of Service	Signalised Level Of Service
1	3.27	0.22	2.291	A	A
2	3.60	0.24	2.660	A	A
3	1.91	0.13	3.052	A	A
4	6.39	0.43	3.565	A	A

Queueing Delay results: (08:45-09:00)

Arm	Queueing Total Delay (Veh-min)	Queueing Rate Of Delay (Veh-min/min)	Average Delay Per Arriving Vehicle (s)	Unsignalised Level Of Service	Signalised Level Of Service
1	2.54	0.17	2.143	A	A
2	2.83	0.19	2.517	A	A
3	1.51	0.10	2.891	A	A
4	4.77	0.32	3.188	A	A

Queueing Delay results: (09:00-09:15)

Arm	Queueing Total Delay (Veh-min)	Queueing Rate Of Delay (Veh-min/min)	Average Delay Per Arriving Vehicle (s)	Unsignalised Level Of Service	Signalised Level Of Service
1	2.02	0.13	2.044	A	A
2	2.28	0.15	2.419	A	A
3	1.21	0.08	2.784	A	A
4	3.70	0.25	2.958	A	A

Queue Variation Results for each time segment

Queue Variation results: (07:45-08:00)

Arm	Mean (Veh)	Q05 (Veh)	Q50 (Veh)	Q90 (Veh)	Q95 (Veh)	Percentile Message	Marker Message	Probability Of Reaching Or Exceeding Marker	Probability Of Exactly Reaching Marker
1	0.13	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
2	0.15	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
3	0.08	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
4	0.24	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A

Queue Variation results: (08:00-08:15)

Arm	Mean (Veh)	Q05 (Veh)	Q50 (Veh)	Q90 (Veh)	Q95 (Veh)	Percentile Message	Marker Message	Probability Of Reaching Or Exceeding Marker	Probability Of Exactly Reaching Marker
1	0.17	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
2	0.19	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
3	0.10	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
4	0.31	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A

Queue Variation results: (08:15-08:30)

Arm	Mean (Veh)	Q05 (Veh)	Q50 (Veh)	Q90 (Veh)	Q95 (Veh)	Percentile Message	Marker Message	Probability Of Reaching Or Exceeding Marker	Probability Of Exactly Reaching Marker
1	0.22	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
2	0.24	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
3	0.13	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
4	0.43	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A

Queue Variation results: (08:30-08:45)

Arm	Mean (Veh)	Q05 (Veh)	Q50 (Veh)	Q90 (Veh)	Q95 (Veh)	Percentile Message	Marker Message	Probability Of Reaching Or Exceeding Marker	Probability Of Exactly Reaching Marker
1	0.22	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
2	0.24	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
3	0.13	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
4	0.43	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A

Queue Variation results: (08:45-09:00)

Arm	Mean (Veh)	Q05 (Veh)	Q50 (Veh)	Q90 (Veh)	Q95 (Veh)	Percentile Message	Marker Message	Probability Of Reaching Or Exceeding Marker	Probability Of Exactly Reaching Marker
1	0.17	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
2	0.19	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
3	0.10	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
4	0.31	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A

Queue Variation results: (09:00-09:15)

Arm	Mean (Veh)	Q05 (Veh)	Q50 (Veh)	Q90 (Veh)	Q95 (Veh)	Percentile Message	Marker Message	Probability Of Reaching Or Exceeding Marker	Probability Of Exactly Reaching Marker
1	0.13	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
2	0.15	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
3	0.08	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
4	0.24	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A

Existing Layout - Testing Year 2023 + Dev, PM Peak

Data Errors and Warnings

Severity	Area	Item	Description
Warning	Geometry	Arm 4 - Roundabout Geometry	Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution.

Analysis Set Details

Name	Roundabout Capacity Model	Description	Include In Report	Use Specific Demand Set(s)	Specific Demand Set(s)	Locked	Network Flow Scaling Factor (%)	Network Capacity Scaling Factor (%)	Reason For Scaling Factors
Existing Layout	ARCADY		✓				100.000	100.000	

Demand Set Details

Name	Scenario Name	Time Period Name	Description	Traffic Profile Type	Model Start Time (HH:mm)	Model Finish Time (HH:mm)	Model Time Period Length (min)	Time Segment Length (min)	Results For Central Hour Only	Single Time Segment Only	Locked	Run Automatically	Use Relationship	Relationship
Testing Year 2023 + Dev, PM Peak	Testing Year 2023 + Dev	PM Peak		ONE HOUR	16:45	18:15	90	15				✓		

Junction Network

Junctions

Name	Junction Type	Arm Order	Grade Separated	Large Roundabout	Do Geometric Delay	Junction Delay (s)	Junction LOS
(untitled)	Roundabout	1,2,3,4				2.92	A

Junction Network Options

Driving Side	Lighting
Left	Normal/unknown

Arms

Arms

Arm	Name	Description
1	Exeter Road (North)	
2	Exeter Road (South)	
3	Bowhill	
4	Swallow Way	

Capacity Options

Arm	Minimum Capacity (PCU/hr)	Maximum Capacity (PCU/hr)	Assume Flat Start Profile	Initial Queue (PCU)
1	0.00	99999.00		0.00
2	0.00	99999.00		0.00
3	0.00	99999.00		0.00
4	0.00	99999.00		0.00

Roundabout Geometry

Arm	V - Approach road half-width (m)	E - Entry width (m)	F - Effective flare length (m)	R - Entry radius (m)	D - Inscribed circle diameter (m)	PHI - Conflict (entry) angle (deg)	Exit Only
1	7.00	8.00	30.00	37.50	30.00	60.00	
2	4.50	8.00	20.00	12.50	30.00	50.00	
3	3.00	8.00	25.00	10.00	30.00	60.00	
4	3.00	7.00	35.00	62.50	30.00	60.00	

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Pedestrian Crossings

Arm	Crossing Type
1	None
2	None
3	None
4	None

Slope / Intercept / Capacity

Roundabout Slope and Intercept used in model

Arm	Enter slope and intercept directly	Entered slope	Entered intercept (PCU/hr)	Final Slope	Final Intercept (PCU/hr)
1		(calculated)	(calculated)	0.735	2200.146
2		(calculated)	(calculated)	0.656	1841.551
3		(calculated)	(calculated)	0.580	1552.365
4		(calculated)	(calculated)	0.630	1669.173

The slope and intercept shown above include any corrections and adjustments.

Traffic Flows

Demand Set Data Options

Default Vehicle Mix	Vehicle Mix Varies Over Time	Vehicle Mix Varies Over Turn	Vehicle Mix Varies Over Entry	Vehicle Mix Source	PCU Factor for a HV (PCU)	Default Turning Proportions	Estimate from entry/exit counts	Turning Proportions Vary Over Time	Turning Proportions Vary Over Turn	Turning Proportions Vary Over Entry
		✓	✓	HV Percentages	2.00				✓	✓

Entry Flows

General Flows Data

Arm	Profile Type	Use Turning Counts	Average Demand Flow (Veh/hr)	Flow Scaling Factor (%)
1	ONE HOUR	✓	391.00	100.000
2	ONE HOUR	✓	504.00	100.000
3	ONE HOUR	✓	149.00	100.000
4	ONE HOUR	✓	221.00	100.000

Turning Proportions

Turning Counts or Proportions (Veh/hr) - Junction 1 (for whole period)

		To			
		1	2	3	4
From	1	0.000	186.000	134.000	71.000
	2	277.000	0.000	3.000	224.000
	3	113.000	5.000	0.000	31.000
	4	52.000	147.000	22.000	0.000

Turning Proportions (Veh) - Junction 1 (for whole period)

		To			
		1	2	3	4
From	1	0.00	0.48	0.34	0.18
	2	0.55	0.00	0.01	0.44
	3	0.76	0.03	0.00	0.21
	4	0.24	0.67	0.10	0.00

Vehicle Mix

Average PCU Per Vehicle - Junction 1 (for whole period)

		To			
		1	2	3	4
From	1	1.000	1.000	1.000	1.000
	2	1.000	1.000	1.000	1.000
	3	1.000	1.000	1.000	1.000
	4	1.000	1.000	1.000	1.000

Heavy Vehicle Percentages - Junction 1 (for whole period)

		To			
		1	2	3	4
From	1	0.000	0.000	0.000	0.000
	2	0.000	0.000	0.000	0.000
	3	0.000	0.000	0.000	0.000
	4	0.000	0.000	0.000	0.000

Results

Results Summary for whole modelled period

Arm	Max RFC	Max Delay (s)	Max Queue (Veh)	Max LOS	Average Demand (Veh/hr)	Total Junction Arrivals (Veh)	Total Queuing Delay (Veh-min)	Average Queuing Delay (s)	Rate Of Queuing Delay (Veh-min/min)	Inclusive Total Queuing Delay (Veh-min)	Inclusive Average Queuing Delay (s)
1	0.21	2.21	0.26	A	358.79	536.18	18.82	2.10	0.21	18.82	2.10
2	0.33	3.21	0.49	A	462.48	693.72	33.90	2.93	0.38	33.91	2.93
3	0.14	3.52	0.16	A	136.73	205.09	11.17	3.27	0.12	11.17	3.27
4	0.17	3.12	0.21	A	202.79	304.19	14.84	2.93	0.16	14.84	2.93

Main Results for each time segment

Main results: (16:45-17:00)

Arm	Total Demand (Veh/hr)	Junction Arrivals (Veh)	Entry Flow (Veh/hr)	Exit Flow (Veh/hr)	Circulating Flow (Veh/hr)	Pedestrian Demand (Ped/hr)	Capacity (Veh/hr)	Saturation Capacity (Veh/hr)	RFC	Start Queue (Veh)	End Queue (Veh)	Delay (s)	LOS
1	294.37	73.59	293.72	331.74	130.60	0.00	2104.15	1670.56	0.140	0.00	0.16	1.988	A
2	379.44	94.86	378.32	253.81	170.51	0.00	1729.66	1207.16	0.219	0.00	0.28	2.661	A
3	112.18	28.04	111.80	119.42	429.40	0.00	1303.21	680.06	0.086	0.00	0.09	3.021	A
4	166.38	41.60	165.88	244.74	296.46	0.00	1482.51	912.34	0.112	0.00	0.13	2.734	A

Main results: (17:00-17:15)

Arm	Total Demand (Veh/hr)	Junction Arrivals (Veh)	Entry Flow (Veh/hr)	Exit Flow (Veh/hr)	Circulating Flow (Veh/hr)	Pedestrian Demand (Ped/hr)	Capacity (Veh/hr)	Saturation Capacity (Veh/hr)	RFC	Start Queue (Veh)	End Queue (Veh)	Delay (s)	LOS
1	351.50	87.88	351.34	397.07	156.32	0.00	2085.24	1670.56	0.169	0.16	0.20	2.076	A
2	453.09	113.27	452.77	303.69	203.97	0.00	1707.70	1207.16	0.265	0.28	0.36	2.868	A
3	133.95	33.49	133.85	142.87	513.87	0.00	1254.19	680.06	0.107	0.09	0.12	3.212	A
4	198.67	49.67	198.54	292.88	354.84	0.00	1445.75	912.34	0.137	0.13	0.16	2.886	A

Main results: (17:15-17:30)

Arm	Total Demand (Veh/hr)	Junction Arrivals (Veh)	Entry Flow (Veh/hr)	Exit Flow (Veh/hr)	Circulating Flow (Veh/hr)	Pedestrian Demand (Ped/hr)	Capacity (Veh/hr)	Saturation Capacity (Veh/hr)	RFC	Start Queue (Veh)	End Queue (Veh)	Delay (s)	LOS
1	430.50	107.62	430.25	486.19	191.41	0.00	2059.45	1670.56	0.209	0.20	0.26	2.209	A
2	554.91	138.73	554.39	371.89	249.78	0.00	1677.63	1207.16	0.331	0.36	0.49	3.203	A
3	164.05	41.01	163.89	174.95	629.21	0.00	1187.26	680.06	0.138	0.12	0.16	3.517	A
4	243.33	60.83	243.12	358.62	434.48	0.00	1395.61	912.34	0.174	0.16	0.21	3.123	A

Main results: (17:30-17:45)

Arm	Total Demand (Veh/hr)	Junction Arrivals (Veh)	Entry Flow (Veh/hr)	Exit Flow (Veh/hr)	Circulating Flow (Veh/hr)	Pedestrian Demand (Ped/hr)	Capacity (Veh/hr)	Saturation Capacity (Veh/hr)	RFC	Start Queue (Veh)	End Queue (Veh)	Delay (s)	LOS
1	430.50	107.62	430.50	486.65	191.58	0.00	2059.33	1670.56	0.209	0.26	0.26	2.209	A
2	554.91	138.73	554.91	372.14	249.93	0.00	1677.53	1207.16	0.331	0.49	0.49	3.206	A
3	164.05	41.01	164.05	175.06	629.78	0.00	1186.94	680.06	0.138	0.16	0.16	3.518	A
4	243.33	60.83	243.32	358.93	434.90	0.00	1395.35	912.34	0.174	0.21	0.21	3.124	A

Main results: (17:45-18:00)

Arm	Total Demand (Veh/hr)	Junction Arrivals (Veh)	Entry Flow (Veh/hr)	Exit Flow (Veh/hr)	Circulating Flow (Veh/hr)	Pedestrian Demand (Ped/hr)	Capacity (Veh/hr)	Saturation Capacity (Veh/hr)	RFC	Start Queue (Veh)	End Queue (Veh)	Delay (s)	LOS
1	351.50	87.88	351.74	397.80	156.58	0.00	2085.05	1670.56	0.169	0.26	0.20	2.078	A
2	453.09	113.27	453.61	304.11	204.22	0.00	1707.54	1207.16	0.265	0.49	0.36	2.873	A
3	133.95	33.49	134.11	143.04	514.78	0.00	1253.67	680.06	0.107	0.16	0.12	3.215	A
4	198.67	49.67	198.88	293.38	355.51	0.00	1445.33	912.34	0.137	0.21	0.16	2.890	A

Main results: (18:00-18:15)

Arm	Total Demand (Veh/hr)	Junction Arrivals (Veh)	Entry Flow (Veh/hr)	Exit Flow (Veh/hr)	Circulating Flow (Veh/hr)	Pedestrian Demand (Ped/hr)	Capacity (Veh/hr)	Saturation Capacity (Veh/hr)	RFC	Start Queue (Veh)	End Queue (Veh)	Delay (s)	LOS
1	294.37	73.59	294.53	333.05	131.10	0.00	2103.78	1670.56	0.140	0.20	0.16	1.991	A
2	379.44	94.86	379.76	254.63	171.00	0.00	1729.34	1207.16	0.219	0.36	0.28	2.667	A
3	112.18	28.04	112.28	119.77	430.98	0.00	1302.29	680.06	0.086	0.12	0.09	3.027	A
4	166.38	41.60	166.51	245.62	297.63	0.00	1481.77	912.34	0.112	0.16	0.13	2.736	A

Queueing Delay Results for each time segment

Queueing Delay results: (16:45-17:00)

Arm	Queueing Total Delay (Veh-min)	Queueing Rate Of Delay (Veh-min/min)	Average Delay Per Arriving Vehicle (s)	Unsignalised Level Of Service	Signalised Level Of Service
1	2.40	0.16	1.988	A	A
2	4.13	0.28	2.661	A	A
3	1.38	0.09	3.021	A	A
4	1.86	0.12	2.734	A	A

Queueing Delay results: (17:00-17:15)

Arm	Queueing Total Delay (Veh-min)	Queueing Rate Of Delay (Veh-min/min)	Average Delay Per Arriving Vehicle (s)	Unsignalised Level Of Service	Signalised Level Of Service
1	3.00	0.20	2.076	A	A
2	5.32	0.35	2.868	A	A
3	1.76	0.12	3.212	A	A
4	2.35	0.16	2.886	A	A

Queueing Delay results: (17:15-17:30)

Arm	Queueing Total Delay (Veh-min)	Queueing Rate Of Delay (Veh-min/min)	Average Delay Per Arriving Vehicle (s)	Unsignalised Level Of Service	Signalised Level Of Service
1	3.91	0.26	2.209	A	A
2	7.26	0.48	3.203	A	A
3	2.36	0.16	3.517	A	A
4	3.11	0.21	3.123	A	A

Queueing Delay results: (17:30-17:45)

Arm	Queueing Total Delay (Veh-min)	Queueing Rate Of Delay (Veh-min/min)	Average Delay Per Arriving Vehicle (s)	Unsignalised Level Of Service	Signalised Level Of Service
1	3.96	0.26	2.209	A	A
2	7.39	0.49	3.206	A	A
3	2.40	0.16	3.518	A	A
4	3.16	0.21	3.124	A	A

Queueing Delay results: (17:45-18:00)

Arm	Queueing Total Delay (Veh-min)	Queueing Rate Of Delay (Veh-min/min)	Average Delay Per Arriving Vehicle (s)	Unsignalised Level Of Service	Signalised Level Of Service
1	3.08	0.21	2.078	A	A
2	5.52	0.37	2.873	A	A
3	1.83	0.12	3.215	A	A
4	2.43	0.16	2.890	A	A

Queueing Delay results: (18:00-18:15)

Arm	Queueing Total Delay (Veh-min)	Queueing Rate Of Delay (Veh-min/min)	Average Delay Per Arriving Vehicle (s)	Unsignalised Level Of Service	Signalised Level Of Service
1	2.47	0.16	1.991	A	A
2	4.29	0.29	2.667	A	A
3	1.44	0.10	3.027	A	A
4	1.93	0.13	2.736	A	A

Queue Variation Results for each time segment

Queue Variation results: (16:45-17:00)

Arm	Mean (Veh)	Q05 (Veh)	Q50 (Veh)	Q90 (Veh)	Q95 (Veh)	Percentile Message	Marker Message	Probability Of Reaching Or Exceeding Marker	Probability Of Exactly Reaching Marker
1	0.16	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
2	0.28	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
3	0.09	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
4	0.13	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A

Queue Variation results: (17:00-17:15)

Arm	Mean (Veh)	Q05 (Veh)	Q50 (Veh)	Q90 (Veh)	Q95 (Veh)	Percentile Message	Marker Message	Probability Of Reaching Or Exceeding Marker	Probability Of Exactly Reaching Marker
1	0.20	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
2	0.36	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
3	0.12	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
4	0.16	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A

Queue Variation results: (17:15-17:30)

Arm	Mean (Veh)	Q05 (Veh)	Q50 (Veh)	Q90 (Veh)	Q95 (Veh)	Percentile Message	Marker Message	Probability Of Reaching Or Exceeding Marker	Probability Of Exactly Reaching Marker
1	0.26	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
2	0.49	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
3	0.16	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
4	0.21	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A

Queue Variation results: (17:30-17:45)

Arm	Mean (Veh)	Q05 (Veh)	Q50 (Veh)	Q90 (Veh)	Q95 (Veh)	Percentile Message	Marker Message	Probability Of Reaching Or Exceeding Marker	Probability Of Exactly Reaching Marker
1	0.26	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
2	0.49	0.00	0.00	0.00	1.00			N/A	N/A
3	0.16	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
4	0.21	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A

Queue Variation results: (17:45-18:00)

Arm	Mean (Veh)	Q05 (Veh)	Q50 (Veh)	Q90 (Veh)	Q95 (Veh)	Percentile Message	Marker Message	Probability Of Reaching Or Exceeding Marker	Probability Of Exactly Reaching Marker
1	0.20	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
2	0.36	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
3	0.12	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
4	0.16	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A

Queue Variation results: (18:00-18:15)

Arm	Mean (Veh)	Q05 (Veh)	Q50 (Veh)	Q90 (Veh)	Q95 (Veh)	Percentile Message	Marker Message	Probability Of Reaching Or Exceeding Marker	Probability Of Exactly Reaching Marker
1	0.16	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
2	0.28	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
3	0.09	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A
4	0.13	N/A	N/A	N/A	N/A	Percentiles could not be calculated. This may be because the mean queue is very small or very big.		N/A	N/A

APPENDIX E

PICADY RESULTS FOR EXETER ROAD/COLEBROOKE LANE (EAST) T-JUNCTION

TRL LIMITED

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CAPACITIES, QUEUES, AND DELAYS AT 3 OR 4-ARM MAJOR/MINOR PRIORITY JUNCTIONS

PICADY 5.1 ANALYSIS PROGRAM
RELEASE 5.0 (JUNE 2010)

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Run with file:-

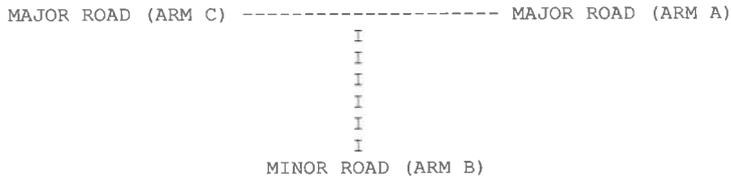
"S:\C9841 - Colebrooke Lane, Cullompton\Calculations\Traffic\Picady\Colebrook Lane-High Street.vpi"
(drive-on-the-left) at 15:17:55 on Tuesday, 25 June 2013

RUN INFORMATION

RUN TITLE : Colebrooke Lane/Exeter Road Junction
LOCATION : Collumpton
DATE : 20/06/13
CLIENT : Genesis Town Planning
ENUMERATOR : sradford [JUBB-9274]
JOB NUMBER : C9841 - Colebrooke Lane, Cullompton
STATUS : Preliminary
DESCRIPTION :

MAJOR/MINOR JUNCTION CAPACITY AND DELAY

INPUT DATA



ARM A IS Exeter Rd South
ARM B IS Colebrook Ln
ARM C IS Exeter Rd North

STREAM LABELLING CONVENTION

STREAM A-B CONTAINS TRAFFIC GOING FROM ARM A TO ARM B
STREAM B-AC CONTAINS TRAFFIC GOING FROM ARM B TO ARM A AND TO ARM C
ETC.

 GEOMETRIC DATA

I	DATA ITEM	I	MINOR ROAD B	I
I	TOTAL MAJOR ROAD CARRIAGEWAY WIDTH	I	(W) 6.00 M.	I
I	CENTRAL RESERVE WIDTH	I	(WCR) 0.00 M.	I
I		I		I
I	MAJOR ROAD RIGHT TURN - WIDTH	I	(WC-B) 2.20 M.	I
I	- VISIBILITY	I	(VC-B) 100.00 M.	I
I	- BLOCKS TRAFFIC (SPACES)	I	YES (0)	I
I		I		I
I	MINOR ROAD - VISIBILITY TO LEFT	I	(VB-C) 25.0 M.	I
I	- VISIBILITY TO RIGHT	I	(VB-A) 28.0 M.	I
I	- LANE 1 WIDTH	I	(WB-C) 3.00 M.	I
I	- LANE 2 WIDTH	I	(WB-A) 0.00 M.	I

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I	Intercept For	Slope For	Opposing	Slope For	Opposing	I
I	STREAM B-C	STREAM	A-C	STREAM	A-B	I
I	641.56		0.25		0.10	I

I	Intercept For	Slope For	Opposing	Slope For	Opposing	Slope For	Opposing	I	
I	STREAM B-A	STREAM	A-C	STREAM	A-B	STREAM	C-A	STREAM	C-B
I	499.45		0.23		0.09		0.14		0.33

I	Intercept For	Slope For	Opposing	Slope For	Opposing	I
I	STREAM C-B	STREAM	A-C	STREAM	A-B	I
I	631.87		0.24		0.24	I

(NB These values do not allow for any site specific corrections)

TRAFFIC DEMAND DATA

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Demand set: Year 2013 Baseline Condition AM Peak

TIME PERIOD BEGINS 07.45 AND ENDS 09.15

LENGTH OF TIME PERIOD - 90 MIN.
 LENGTH OF TIME SEGMENT - 15 MIN.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	ARM	I	NUMBER OF MINUTES FROM START WHEN	I	RATE OF FLOW (VEH/MIN)	I
I		I	FLOW STARTS	I	BEFORE	I
I		I	TOP OF PEAK	I	AT TOP	I
I		I	IS REACHED	I	OF PEAK	I
I		I	FALLING	I	PEAK	I
I		I		I		I
I	ARM A	I	15.00	I	45.00	I
I	ARM B	I	15.00	I	45.00	I
I	ARM C	I	15.00	I	45.00	I
I		I		I	75.00	I
I		I		I	4.57	I
I		I		I	6.86	I
I		I		I	4.57	I
I		I		I	1.33	I
I		I		I	1.99	I
I		I		I	1.33	I
I		I		I	3.90	I
I		I		I	5.85	I
I		I		I	3.90	I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.45-09.00										I
I	B-AC	1.59	7.38	0.215		0.38	0.28	4.3		0.17	I
I	C-AB	0.86	11.11	0.077		0.21	0.14	2.2		0.10	I
I	C-A	3.81									I
I	A-B	0.15									I
I	A-C	5.33									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	09.00-09.15										I
I	B-AC	1.33	7.66	0.174		0.28	0.21	3.3		0.16	I
I	C-AB	0.65	10.81	0.060		0.14	0.10	1.5		0.10	I
I	C-A	3.26									I
I	A-B	0.13									I
I	A-C	4.47									I

WARNING NO MARGINAL ANALYSIS OF CAPACITIES AS MAJOR ROAD BLOCKING MAY OCCUR

QUEUE FOR STREAM B-AC

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
08.00	0.2
08.15	0.3
08.30	0.4
08.45	0.4
09.00	0.3
09.15	0.2

QUEUE FOR STREAM C-AB

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
08.00	0.1
08.15	0.1
08.30	0.2
08.45	0.2
09.00	0.1
09.15	0.1

 QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I
I	I	I	I	I	* DELAY *	I	* DELAY *	I
I	I	I	I	I	I	I	I	I
I	I	(VEH)	(VEH/H)	(MIN)	(MIN/VEH)	(MIN)	(MIN/VEH)	I
I	B-AC	I 145.9	I 97.3	I 25.7	I 0.18	I 25.7	I 0.18	I
I	C-AB	I 80.6	I 53.7	I 13.5	I 0.17	I 13.5	I 0.17	I
I	C-A	I 348.9	I 232.6	I	I	I	I	I
I	A-B	I 13.8	I 9.2	I	I	I	I	I
I	A-C	I 490.0	I 326.7	I	I	I	I	I
I	ALL	I 1079.1	I 719.4	I 39.1	I 0.04	I 39.2	I 0.04	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

*****END OF RUN*****

 SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I	Intercept	For Slope	For Opposing	Slope	For Opposing	I
I	STREAM	B-C	STREAM	A-C	STREAM	A-B
I	641.56		0.25		0.10	I

I	Intercept	For Slope	For Opposing	Slope	For Opposing	Slope	For Opposing	I
I	STREAM	B-A	STREAM	A-C	STREAM	A-B	STREAM	C-A
I	I	I	I	I	I	I	I	I
I	499.45		0.23		0.09		0.14	0.33

I	Intercept	For Slope	For Opposing	Slope	For Opposing	I
I	STREAM	C-B	STREAM	A-C	STREAM	A-B
I	631.87		0.24		0.24	I

(NB These values do not allow for any site specific corrections)

 TRAFFIC DEMAND DATA

I	ARM	I	FLOW SCALE(%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Demand set: Year 2013 Baseline Condition PM Peak

TIME PERIOD BEGINS 16.45 AND ENDS 18.15

LENGTH OF TIME PERIOD - 90 MIN.
 LENGTH OF TIME SEGMENT - 15 MIN.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	ARM	I	NUMBER OF MINUTES FROM START WHEN	I	RATE OF FLOW (VEH/MIN)	I
I	I	I	FLOW STARTS	I	BEFORE	I
I	I	I	TO RISE	I	AT TOP	I
I	I	I	IS REACHED	I	OF PEAK	I
I	I	I	FALLING	I	PEAK	I
I	ARM A	I	15.00	I	4.96	I
I	ARM B	I	15.00	I	0.57	I
I	ARM C	I	15.00	I	5.19	I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.45-18.00										I
I	B-AC	0.69	7.32	0.094		0.14	0.11	1.6		0.15	I
I	C-AB	2.06	11.66	0.177		0.54	0.36	5.4		0.10	I
I	C-A	4.16									I
I	A-B	0.42									I
I	A-C	5.53									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	18.00-18.15										I
I	B-AC	0.58	7.63	0.076		0.11	0.08	1.3		0.14	I
I	C-AB	1.57	11.29	0.139		0.36	0.26	3.9		0.10	I
I	C-A	3.64									I
I	A-B	0.35									I
I	A-C	4.63									I

WARNING NO MARGINAL ANALYSIS OF CAPACITIES AS MAJOR ROAD BLOCKING MAY OCCUR

QUEUE FOR STREAM B-AC

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
17.00	0.1
17.15	0.1
17.30	0.1
17.45	0.1
18.00	0.1
18.15	0.1

QUEUE FOR STREAM C-AB

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
17.00	0.2
17.15	0.3
17.30	0.5 *
17.45	0.5 *
18.00	0.4
18.15	0.3

 QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I
I	I	I	I	I	* DELAY *	I	* DELAY *	I
I	I	I	I	I	I	I	I	I
I	I	(VEH)	(VEH/H)	I	(MIN)	(MIN/VEH)	(MIN)	(MIN/VEH)
I	B-AC	I 63.3	I 42.2	I	9.7	I 0.15	I 9.7	I 0.15
I	C-AB	I 194.7	I 129.8	I	34.3	I 0.18	I 34.3	I 0.18
I	C-A	I 376.5	I 251.0	I		I	I	I
I	A-B	I 38.5	I 25.7	I		I	I	I
I	A-C	I 507.9	I 338.6	I		I	I	I
I	ALL	I 1181.0	I 787.3	I	44.0	I 0.04	I 44.0	I 0.04

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

*****END OF RUN*****

 SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I	Intercept	For Slope	For Opposing	Slope	For Opposing	I
I	STREAM	B-C	STREAM	A-C	STREAM	A-B
I	641.56		0.25		0.10	I

I	Intercept	For Slope	For Opposing	Slope	For Opposing	Slope	For Opposing	I
I	STREAM	B-A	STREAM	A-C	STREAM	A-B	STREAM	C-A
I							STREAM	C-B
I	499.45		0.23		0.09		0.14	0.33

I	Intercept	For Slope	For Opposing	Slope	For Opposing	I
I	STREAM	C-B	STREAM	A-C	STREAM	A-B
I	631.87		0.24		0.24	I

(NB These values do not allow for any site specific corrections)

 TRAFFIC DEMAND DATA

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Demand set: Year 2023 + Dev AM Peak

TIME PERIOD BEGINS 07.45 AND ENDS 09.15

LENGTH OF TIME PERIOD - 90 MIN.
 LENGTH OF TIME SEGMENT - 15 MIN.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	ARM	I	NUMBER OF MINUTES FROM START WHEN	I	RATE OF FLOW (VEH/MIN)	I
I	I	I	FLOW STARTS	I	BEFORE	I
I	I	I	TO RISE	I	AT TOP	I
I	I	I	IS REACHED	I	OF PEAK	I
I	I	I	FALLING	I	PEAK	I
I	A	I	15.00	I	5.16	I
I	B	I	15.00	I	3.81	I
I	C	I	15.00	I	5.19	I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.45-09.00										I
I	B-AC	4.57	7.68	0.595		3.11	1.54	25.4		0.34	I
I	C-AB	2.63	11.35	0.231		0.72	0.47	7.1		0.12	I
I	C-A	3.59									I
I	A-B	0.16									I
I	A-C	6.02									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	09.00-09.15										I
I	B-AC	3.83	7.98	0.480		1.54	0.95	15.1		0.25	I
I	C-AB	2.01	11.03	0.182		0.47	0.33	5.0		0.11	I
I	C-A	3.20									I
I	A-B	0.14									I
I	A-C	5.04									I

WARNING NO MARGINAL ANALYSIS OF CAPACITIES AS MAJOR ROAD BLOCKING MAY OCCUR

QUEUE FOR STREAM B-AC

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
08.00	0.9	*
08.15	1.4	*
08.30	2.9	***
08.45	3.1	***
09.00	1.5	**
09.15	0.9	*

QUEUE FOR STREAM C-AB

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
08.00	0.3	
08.15	0.5	
08.30	0.7	*
08.45	0.7	*
09.00	0.5	
09.15	0.3	

 QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I
I	I	I	I	I	* DELAY *	I	* DELAY *	I
I	I	I	I	I	I	I	I	I
I	I	(VEH)	(VEH/H)	(MIN)	(MIN/VEH)	(MIN)	(MIN/VEH)	I
I	B-AC	I 419.8	I 279.9	I 156.2	I 0.37	I 156.2	I 0.37	I
I	C-AB	I 249.0	I 166.0	I 45.5	I 0.18	I 45.5	I 0.18	I
I	C-A	I 322.2	I 214.8	I	I	I	I	I
I	A-B	I 15.1	I 10.1	I	I	I	I	I
I	A-C	I 553.3	I 368.9	I	I	I	I	I
I	ALL	I 1559.5	I 1039.7	I 201.7	I 0.13	I 201.7	I 0.13	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD
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 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

*****END OF RUN*****

 SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I	Intercept	For Slope	For Opposing	Slope	For Opposing	I
I	STREAM	B-C	STREAM A-C	STREAM	A-B	I
I	641.56		0.25	0.10		I

I	Intercept	For Slope	For Opposing	Slope	For Opposing	Slope	For Opposing	I	
I	STREAM	B-A	STREAM A-C	STREAM	A-B	STREAM	C-A	STREAM	C-B
I	499.45		0.23	0.09		0.14		0.33	I

I	Intercept	For Slope	For Opposing	Slope	For Opposing	I
I	STREAM	C-B	STREAM A-C	STREAM	A-B	I
I	631.87		0.24	0.24		I

(NB These values do not allow for any site specific corrections)

 TRAFFIC DEMAND DATA

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Demand set: Year 2023 + Dev PM Peak

TIME PERIOD BEGINS 16.45 AND ENDS 18.15

LENGTH OF TIME PERIOD - 90 MIN.
 LENGTH OF TIME SEGMENT - 15 MIN.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	ARM	I	NUMBER OF MINUTES FROM START WHEN	I	RATE OF FLOW (VEH/MIN)	I								
I	I	I	FLOW STARTS	I	TOP OF PEAK	I								
I	I	I	TO RISE	I	IS REACHED	I								
I	I	I	I	I	FALLING	I								
I	I	I	I	I	BEFORE	I								
I	I	I	I	I	AT TOP	I								
I	I	I	I	I	AFTER	I								
I	I	I	I	I	PEAK	I								
I	I	I	I	I	OF PEAK	I								
I	I	I	I	I	PEAK	I								
I	ARM A	I	15.00	I	45.00	I	75.00	I	5.61	I	8.42	I	5.61	I
I	ARM B	I	15.00	I	45.00	I	75.00	I	1.94	I	2.91	I	1.94	I
I	ARM C	I	15.00	I	45.00	I	75.00	I	7.50	I	11.25	I	7.50	I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.45-18.00										I
I	B-AC	2.32	7.60	0.306		0.66	0.45	7.0		0.19	I
I	C-AB	6.37	12.07	0.528		3.78	1.73	27.5		0.19	I
I	C-A	2.62									I
I	A-B	0.48									I
I	A-C	6.25									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	18.00-18.15										I
I	B-AC	1.94	7.94	0.245		0.45	0.33	5.1		0.17	I
I	C-AB	4.73	11.57	0.409		1.73	1.01	15.4		0.15	I
I	C-A	2.80									I
I	A-B	0.40									I
I	A-C	5.23									I

WARNING NO MARGINAL ANALYSIS OF CAPACITIES AS MAJOR ROAD BLOCKING MAY OCCUR

QUEUE FOR STREAM B-AC

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
17.00	0.3	
17.15	0.4	
17.30	0.7	*
17.45	0.7	*
18.00	0.4	
18.15	0.3	

QUEUE FOR STREAM C-AB

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
17.00	1.0	*
17.15	1.6	**
17.30	3.6	****
17.45	3.8	****
18.00	1.7	**
18.15	1.0	*

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I
I	I	I	I	I	* DELAY *	I	* DELAY *	I
I	I	I	(VEH)	I	(MIN)	I	(MIN)	I
I	I	I	(VEH/H)	I	(MIN/VEH)	I	(MIN/VEH)	I
I	B-AC	I	213.3	I	42.1	I	0.20	I
I	C-AB	I	603.9	I	192.7	I	0.32	I
I	C-A	I	221.9	I		I		I
I	A-B	I	44.0	I		I		I
I	A-C	I	574.0	I		I		I
I	ALL	I	1657.2	I	234.7	I	0.14	I
			1104.8				234.8	
								0.14

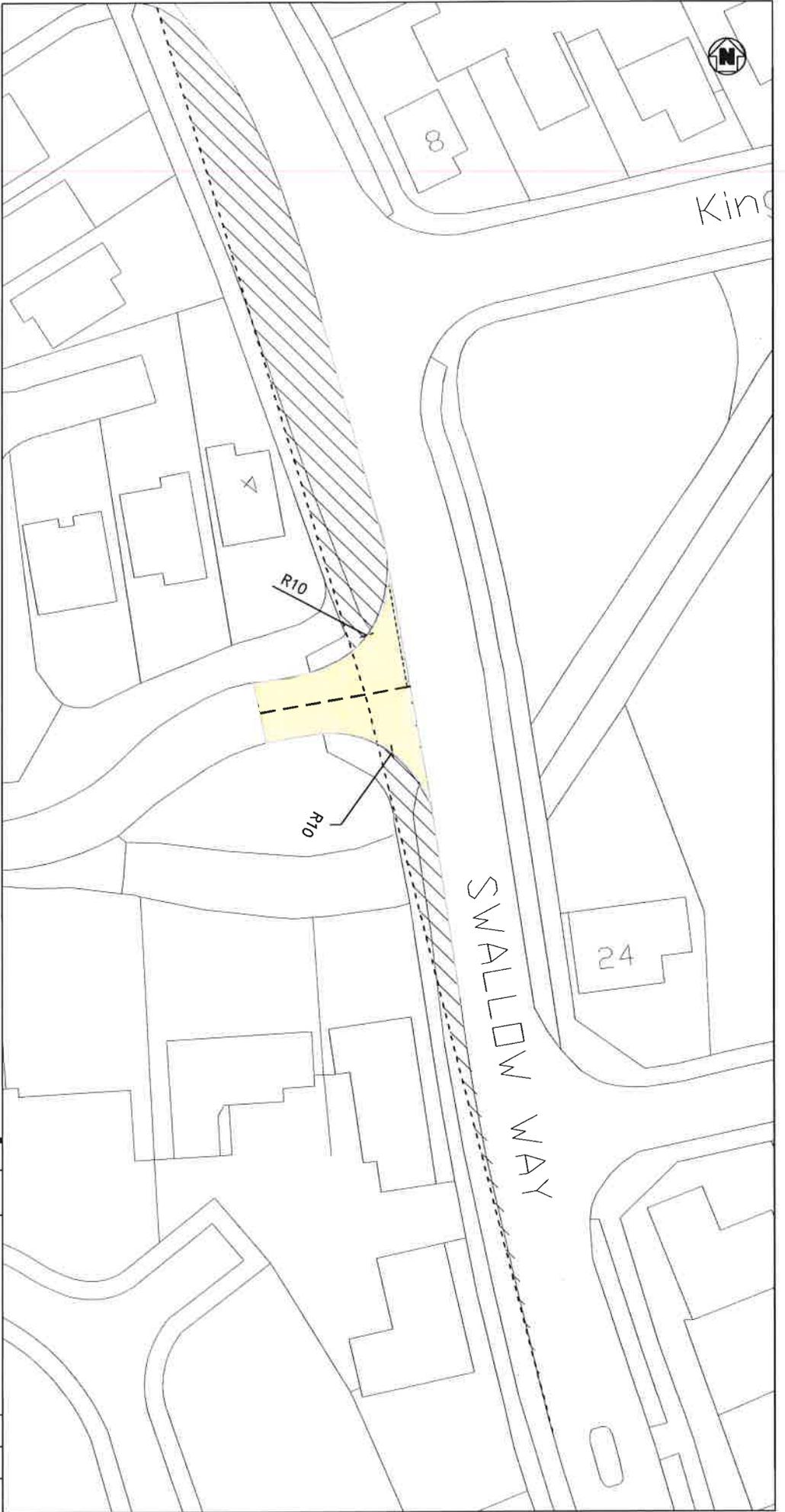
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 WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS
 A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

*****END OF RUN*****

==== end of file =====

APPENDIX F

CONCEPT HIGHWAY ACCESS



- KEY**
-  PROPOSED CARRIAGEWAY
 -  4.5 x 7.5m VISIBILITY SPLAY

- Issue Status**
- CONCEPT
 - PRELIMINARY
 - TENDER
 - CONSTRUCTION
 - H&S FILE ISSUE



BRISTOL FARNBOROUGH CARDIFF PLYMOUTH

Job
COLEBROOKE LANE
CULLOMPTON
SWALLOW WAY/COLEBROOKE LANE
JUNCTION IMPROVEMENT

Detail
 Scale A3
 1:250 Job No. C9841 Dwg No. H001 Rev. 02

REV	DATE	DESCRIPTION	MP	SR	MM
A	25.06.13	FIRST ISSUE			

TRL LIMITED

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CAPACITIES, QUEUES, AND DELAYS AT 3 OR 4-ARM MAJOR/MINOR PRIORITY JUNCTIONS

PICADY 5.1 ANALYSIS PROGRAM
RELEASE 5.0 (JUNE 2010)

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Run with file:-

"S:\C9841 - Colebrooke Lane, Cullompton\Calculations\Traffic\Picady\Colebrook Lane-Swallow Way.vpi"
(drive-on-the-left) at 10:56:35 on Thursday, 27 June 2013

RUN INFORMATION

RUN TITLE : Colebrook Lane/Exeter Road Junction
LOCATION : Cullompton
DATE : 20/06/13
CLIENT : Genesis Town Planning
ENUMERATOR : sradford [JUBB-9274]
JOB NUMBER : C9841 - Colebrooke Lane, Cullompton
STATUS : Preliminary
DESCRIPTION :

MAJOR/MINOR JUNCTION CAPACITY AND DELAY

INPUT DATA

MAJOR ROAD (ARM C) ----- MAJOR ROAD (ARM A)
I
I
I
I
I
I
MINOR ROAD (ARM B)

ARM A IS Swallow Way South
ARM B IS Colebrooke Ln
ARM C IS Swallow Way North

STREAM LABELLING CONVENTION

STREAM A-B CONTAINS TRAFFIC GOING FROM ARM A TO ARM B
STREAM B-AC CONTAINS TRAFFIC GOING FROM ARM B TO ARM A AND TO ARM C
ETC.

 GEOMETRIC DATA

I	DATA ITEM	I	MINOR ROAD B	I
I	TOTAL MAJOR ROAD CARRIAGEWAY WIDTH	I	(W) 7.00 M.	I
I	CENTRAL RESERVE WIDTH	I	(WCR) 0.00 M.	I
I		I		I
I	MAJOR ROAD RIGHT TURN - WIDTH	I	(WC-B) 2.20 M.	I
I	- VISIBILITY	I	(VC-B) 100.00 M.	I
I	- BLOCKS TRAFFIC (SPACES)	I	YES (0)	I
I		I		I
I	MINOR ROAD - VISIBILITY TO LEFT	I	(VB-C) 25.0 M.	I
I	- VISIBILITY TO RIGHT	I	(VB-A) 33.0 M.	I
I	- LANE 1 WIDTH	I	(WB-C) 3.00 M.	I
I	- LANE 2 WIDTH	I	(WB-A) 0.00 M.	I

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I	Intercept For	Slope For	Opposing	Slope For	Opposing	I
I	STREAM B-C	STREAM	A-C	STREAM	A-B	I
I	644.71		0.24		0.09	I

I	Intercept For	Slope For	Opposing	Slope For	Opposing	Slope For	Opposing	Slope For	Opposing	I
I	STREAM B-A	STREAM	A-C	STREAM	A-B	STREAM	C-A	STREAM	C-B	I
I	501.90		0.22		0.09		0.14		0.32	I

I	Intercept For	Slope For	Opposing	Slope For	Opposing	I
I	STREAM C-B	STREAM	A-C	STREAM	A-B	I
I	631.87		0.23		0.23	I

(NB These values do not allow for any site specific corrections)

TRAFFIC DEMAND DATA

I	ARM	I	FLOW SCALE(%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Demand set: Year 2023 + Dev AM Peak

TIME PERIOD BEGINS 07.45 AND ENDS 09.15

LENGTH OF TIME PERIOD - 90 MIN.
 LENGTH OF TIME SEGMENT - 15 MIN.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	ARM	I	NUMBER OF MINUTES FROM START WHEN FLOW STARTS	I	TOP OF PEAK IS REACHED	I	RATE OF FLOW (VEH/MIN) BEFORE PEAK	I	AT TOP OF PEAK	I	AFTER PEAK	I	
I	ARM	A	15.00	I	45.00	I	75.00	I	1.69	I	2.53	I	1.69
I	ARM	B	15.00	I	45.00	I	75.00	I	4.61	I	6.92	I	4.61
I	ARM	C	15.00	I	45.00	I	75.00	I	4.32	I	6.49	I	4.32

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	08.45-09.00										I
I	B-AC	5.53	8.61	0.642		3.80	1.90	31.4		0.35	I
I	C-AB	1.81	12.35	0.146		0.35	0.25	3.8		0.10	I
I	C-A	3.38									I
I	A-B	0.66									I
I	A-C	1.36									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	09.00-09.15										I
I	B-AC	4.63	8.76	0.529		1.90	1.16	18.5		0.25	I
I	C-AB	1.42	12.00	0.119		0.25	0.19	2.9		0.09	I
I	C-A	2.92									I
I	A-B	0.55									I
I	A-C	1.14									I

WARNING NO MARGINAL ANALYSIS OF CAPACITIES AS MAJOR ROAD BLOCKING MAY OCCUR

QUEUE FOR STREAM B-AC

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
08.00	1.1	*
08.15	1.7	**
08.30	3.5	****
08.45	3.8	****
09.00	1.9	**
09.15	1.2	*

QUEUE FOR STREAM C-AB

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE
08.00	0.2
08.15	0.2
08.30	0.3
08.45	0.3
09.00	0.3
09.15	0.2

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND	I	* QUEUEING *	I	* INCLUSIVE QUEUEING *	I	
I	I	I	I	I	* DELAY *	I	* DELAY *	I	
I	I	(VEH)	(VEH/H)	I	(MIN)	(MIN/VEH)	(MIN)	(MIN/VEH)	
I	B-AC	I	507.9	I	338.6	I	190.0	I	0.37
I	C-AB	I	169.1	I	112.8	I	23.5	I	0.14
I	C-A	I	307.1	I	204.7	I	I	I	I
I	A-B	I	60.6	I	40.4	I	I	I	I
I	A-C	I	125.3	I	83.5	I	I	I	I
I	ALL	I	1170.0	I	780.0	I	213.6	I	0.18

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

*****END OF RUN*****

.SLOPES AND INTERCEPT

(NB:Streams may be combined, in which case capacity will be adjusted)

I	Intercept	For	Slope	For	Opposing	Slope	For	Opposing
I	STREAM	B-C	STREAM	A-C	STREAM	A-B	STREAM	A-B
I	644.71		0.24		0.09			

I	Intercept	For	Slope	For	Opposing	Slope	For	Opposing	Slope	For	Opposing
I	STREAM	B-A	STREAM	A-C	STREAM	A-B	STREAM	C-A	STREAM	C-B	STREAM
I	501.90		0.22		0.09		0.14		0.32		

I	Intercept	For	Slope	For	Opposing	Slope	For	Opposing
I	STREAM	C-B	STREAM	A-C	STREAM	A-B	STREAM	A-B
I	631.87		0.23		0.23			

(NB These values do not allow for any site specific corrections)

TRAFFIC DEMAND DATA

I	ARM	I	FLOW SCALE (%)	I
I	A	I	100	I
I	B	I	100	I
I	C	I	100	I

Demand set: Year 2023 + Dev PM Peak

TIME PERIOD BEGINS 16.45 AND ENDS 18.15

LENGTH OF TIME PERIOD - 90 MIN.
 LENGTH OF TIME SEGMENT - 15 MIN.

DEMAND FLOW PROFILES ARE SYNTHESISED FROM TURNING COUNT DATA

I	ARM	I	NUMBER OF MINUTES FROM START WHEN FLOW STARTS	I	TOP OF PEAK IS REACHED	I	FLOW STOPS FALLING	I	RATE OF FLOW (VEH/MIN) BEFORE PEAK	I	AT TOP OF PEAK	I	AFTER PEAK
I	ARM A	I	15.00	I	45.00	I	75.00	I	4.09	I	6.13	I	4.09
I	ARM B	I	15.00	I	45.00	I	75.00	I	2.59	I	3.88	I	2.59
I	ARM C	I	15.00	I	45.00	I	75.00	I	3.99	I	5.98	I	3.99

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	17.45-18.00										I
I	B-AC	3.10	7.95	0.390		0.99	0.65	10.3		0.21	I
I	C-AB	2.93	10.81	0.271		0.68	0.47	7.0		0.13	I
I	C-A	1.85									I
I	A-B	1.36									I
I	A-C	3.54									I

I	TIME	DEMAND (VEH/MIN)	CAPACITY (VEH/MIN)	DEMAND/ CAPACITY (RFC)	PEDESTRIAN FLOW (PEDS/MIN)	START QUEUE (VEHS)	END QUEUE (VEHS)	DELAY (VEH.MIN/ TIME SEGMENT)	GEOMETRIC DELAY (VEH.MIN/ TIME SEGMENT)	AVERAGE DELAY PER ARRIVING VEHICLE (MIN)	I
I	18.00-18.15										I
I	B-AC	2.60	8.21	0.316		0.65	0.47	7.3		0.18	I
I	C-AB	2.34	10.70	0.219		0.47	0.34	5.1		0.12	I
I	C-A	1.66									I
I	A-B	1.14									I
I	A-C	2.96									I

WARNING NO MARGINAL ANALYSIS OF CAPACITIES AS MAJOR ROAD BLOCKING MAY OCCUR

QUEUE FOR STREAM B-AC

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
17.00	0.5	
17.15	0.6	*
17.30	1.0	*
17.45	1.0	*
18.00	0.7	*
18.15	0.5	

QUEUE FOR STREAM C-AB

TIME SEGMENT ENDING	NO. OF VEHICLES IN QUEUE	
17.00	0.3	
17.15	0.5	
17.30	0.7	*
17.45	0.7	*
18.00	0.5	
18.15	0.3	

QUEUEING DELAY INFORMATION OVER WHOLE PERIOD

I	STREAM	I	TOTAL DEMAND (VEH)	I	189.9 (VEH/H)	I	* QUEUEING * * DELAY *	I	* INCLUSIVE QUEUEING * * DELAY *	I
I		I	(VEH)	I	(VEH/H)	I	(MIN)	I	(MIN/VEH)	I
I	B-AC	I	284.9	I	189.9	I	61.6	I	0.22	I
I	C-AB	I	272.8	I	181.8	I	44.3	I	0.16	I
I	C-A	I	166.3	I	110.9	I		I		I
I	A-B	I	125.3	I	83.5	I		I		I
I	A-C	I	324.8	I	216.6	I		I		I
I	ALL	I	1174.1	I	782.7	I	105.9	I	0.09	I

* DELAY IS THAT OCCURRING ONLY WITHIN THE TIME PERIOD
 * INCLUSIVE DELAY INCLUDES DELAY SUFFERED BY VEHICLES WHICH ARE STILL QUEUEING AFTER THE END OF THE TIME PERIOD
 * THESE WILL ONLY BE SIGNIFICANTLY DIFFERENT IF THERE IS A LARGE QUEUE REMAINING AT THE END OF THE TIME PERIOD.

*****END OF RUN*****

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